



Kennedy Creek – Flooding Investigation



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GLOSSARY OF TERMS

Probability (AEP)

(AHD)

(ARI)

Annual Exceedance Refers to the probability or risk of a rainfall event of a given magnitude (intensity and duration) occurring or being exceeded in any given year. A

90% AEP event has a high probability of occurring or being exceeded; it would occur quite often and would be a relatively minor rainfall event. A 1% AEP event has a low probability of occurrence or being exceeded; it

would be rare but it would be likely to cause extensive damage.

Australian Height Datum A common national surface level datum approximately corresponding to

mean sea level. Introduced in 1971 to eventually supersede all earlier

datums.

Average Recurrence Interval Refers to the average time interval between a given flood magnitude

> occurring or being exceeded. A 10 year ARI flood is expected to be exceeded on average once every 10 years. A 100 year ARI flood is expected to be exceeded on average once every 100 years. The AEP is the

ARI expressed as a percentage.

Cadastre, cadastral base Information in map or digital form showing the extent and usage of land,

including streets, lot boundaries, water courses etc.

Catchment The area draining to a site. Generally relates to a particular location and

may include the catchments of tributary streams as well as the main

stream.

Design flood A significant event to be considered in the design process; various works

within the floodplain may have different design standards. A design flood

will generally have a nominated AEP or ARI (see above).

Discharge The rate of flow of water measured in terms of volume over time. It is to

be distinguished from the speed or velocity of flow, which is a measure of

how fast the water is moving rather than how much is moving.

Flood Relatively high stream flow which overtops the natural or artificial banks

> in any part of a stream, river, estuary, lake or dam, and/or overland runoff before entering a watercourse and/or coastal inundation resulting from

elevated sea levels and/or waves overtopping coastline defences.

Flood damage The tangible and intangible costs of flooding.

Flood hazard Potential risk to life and limb caused by flooding. Flood hazard combines

the flood depth and velocity.

Flood mitigation A series of works to prevent or reduce the impact of flooding. This

includes structural options such as levees and non-structural options such

as planning schemes and flood warning systems.

Floodplain Area of land which is subject to inundation by floods up to the probable

maximum flood event, i.e. flood prone land.

Flood storages Those parts of the floodplain that are important for the temporary storage,

of floodwaters during the passage of a flood.

Freeboard A factor of safety above design flood levels typically used in relation to the

setting of floor levels or crest heights of flood levees. It is usually expressed

as a height above the level of the design flood event.

Geographical information

A system of software and procedures designed to support the management, manipulation, analysis and display of spatially referenced systems (GIS)

data.



Hydraulics The term given to the study of water flow in a river, channel or pipe, in

particular, the evaluation of flow parameters such as stage and velocity.

Hydrograph A graph that shows how the discharge changes with time at any particular

location.

Hydrology The term given to the study of the rainfall and runoff process as it relates

to the derivation of hydrographs for given floods.

Intensity frequency duration

(IFD) analysis

Statistical analysis of rainfall, describing the rainfall intensity (mm/hr), frequency (probability measured by the AEP), duration (hrs). This analysis

is used to generate design rainfall estimates.

TUFLOW A hydraulic modelling tool used in this study to simulate the flow of flood

water through the floodplain. The model uses numerical equations to

describe the water movement.

Ortho-photography Aerial photography which has been adjusted to account for topography.

Distance measures on the ortho-photography are true distances on the

ground.

Peak flow The maximum discharge occurring during a flood event.

Probability A statistical measure of the expected frequency or occurrence of flooding.

For a fuller explanation see Average Recurrence Interval.

Risk Chance of something happening that will have an impact. It is measured in

terms of consequence and likelihood. For this study, it is the likelihood of consequences arising from the interaction of floods, communities and the

environment.

RORB A hydrological modelling tool used in this study to calculate the runoff

generated for design rainfall events.

Runoff The amount of rainfall that actually ends up as stream or pipe flow, also

known as rainfall excess.

Stage Equivalent to 'water level'. Both are measured with reference to a specified

datum.

Stage hydrograph A graph that shows how the water level changes with time. It must be

referenced to a particular location and datum.

Topography A surface which defines the ground level of a chosen area.



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1. INTRODUCTION

In late 2013 Benalla Rural City Council (BRCC) commissioned several investigations into drainage related matters in Benalla. Water Technology undertook an investigation into capacity constraints in the Eastern Main Drain and available mitigation options. A spin off from this original work was a high level investigation into flooding in the Kennedy Creek catchment. Drivers for the this study were existing flooding problems (Sydney Road) and the requirement for flooding in the land the north east of the town currently zoned industrial to be better understood.

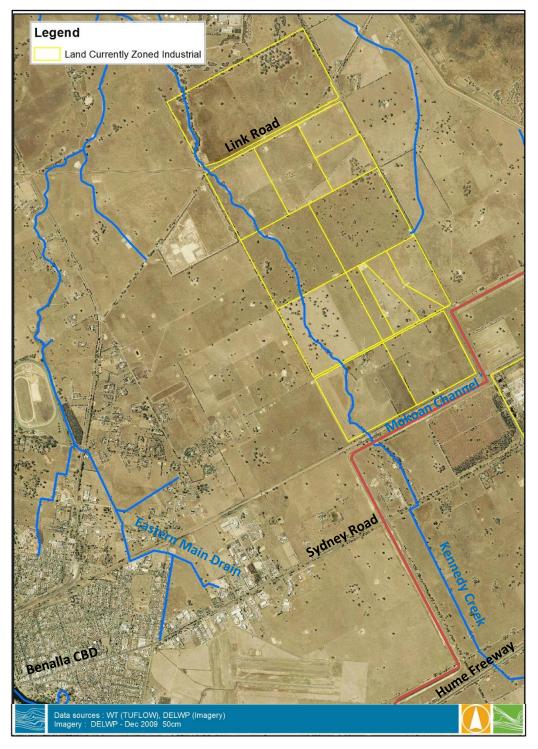


Figure 1-1 Key features inside the Kennedy creek study area



2. SCOPE OF WORKS

To achieve the project objectives the following key tasks were undertaken:

- Gather and review available data;
- Scope survey to fill data gaps;
- Construct hydrological (RORB) model of the Kennedy Creek catchment and produce results from the 5 and 100 year Annual Recurrence Interval (ARI) flood events for peak durations;
- Construct a TUFLOW hydraulic model of the subject site;
- Run the TUFLOW model for the 5 and 100 year ARI events;
- Assess results and provide maps showing the peak flood extents across the 5 and 100 year ARI events. Provide maps showing peak depths, velocities and flood hazard for the 100 year ARI event.
- Provide clear and concise reporting of the methodology and results;
- Provide recommendations on potential mitigation options and meet with Council to discuss;
 and,
- Test mitigation options (max 2) in the hydraulic model and provide flood depth and difference plots.

3. AVAILABLE INFORMATION REVIEW

3.1 Data Collation

Several different datasets were made available for review at the beginning of the project. A preliminary review of this data was used to identify data gaps and scope initial survey requirements. Key data collated included:

Industry Standards:

- Infrastructure Design Manual (IDM) V4.2 (IDM Board 2013);
- Urban Stormwater Best Practice Environmental Management Guidelines (CSIRO 2006);
- Best Practice Principles for Floodplain Management in Australia (CSIRO 2000);
- Melbourne Water Corporation Flood Mapping Projects Guidelines and Technical Specifications (Nov, 2012);
- 2D Modelling Guidelines for Melbourne Water Version 1 (Mar 2011); and,
- Australian Standard AS3500.3 Stormwater Drainage.

Spatial data:

- Light Detection and Ranging (LiDAR) topographic data (DEPI);
- Satellite derived digital elevation data (10 m & 20 m resolution) (DEPI);
- Aerial imagery 2009 (DEPI);
- Cadastral and Planning layers (VicMap data) (DEPI); and,
- Council GIS data including drainage focused data such as pit and pipe information.

Historic Flood information:

- Flood imagery from 1993 (GBCMA);
- Estimated flood levels 1993 (GBCMA); and,
- Broken River Catchment Floods October 1993 (Volume 4) Dept. of Conservation & Natural Resources (report by Hydrotechnology)



3.1.1 Historic Flooding Information

To our knowledge no specific study into flooding in the Kennedy Creek catchment has ever been undertaken. As part of the desktop review, Water Technology reviewed a report into the 1993 flood event. Again no specific commentary about flooding in Kennedy Creek was noted. The image shown in Figure 3-1 is extracted from this report. It covers a small portion of the study area. Flooding in 1993 has been estimated to be a 1 in 100 year ARI.

Six 1993 flood levels inside the study area were found in the Victorian Flood Database. Their locations and levels are shown in Figure 3-2.

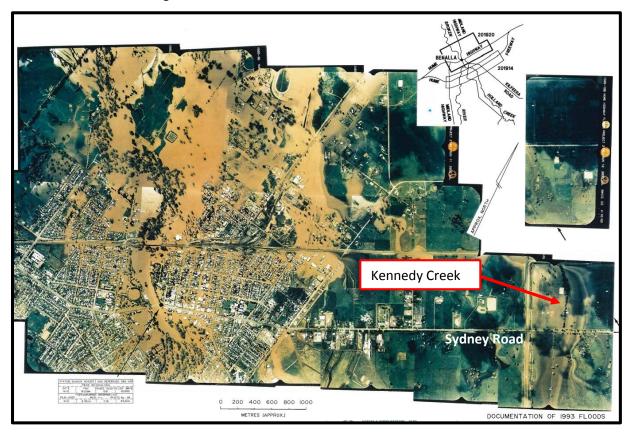


Figure 3-1 1993 Flood mosaic image (Dept. of Conservation & Natural Resources 1993)



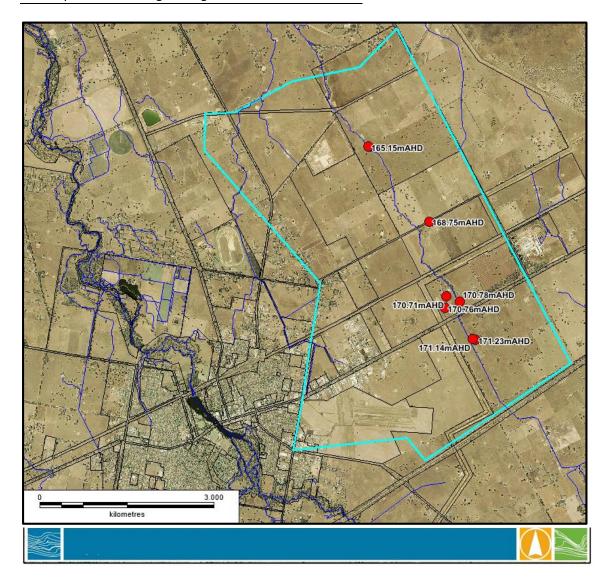


Figure 3-2 1993 Flood Levels (Victorian Flood Database)

3.1.2 Data Gaps

The main data gaps identified were survey related. Water Technology worked closely with EDM group and BRCC to collect the most economical and accurate survey to close the data gaps.

Features / crossings

10 crossing features were surveyed by EDM group. A section of these are depicted in Figure 3-3. Each of these crossings where included as 1D elements in the hydraulic model. Crossing geometry and elevation data was included in the hydraulic model to ensure hydraulic capacities could be resolved in the TUFLOW model.



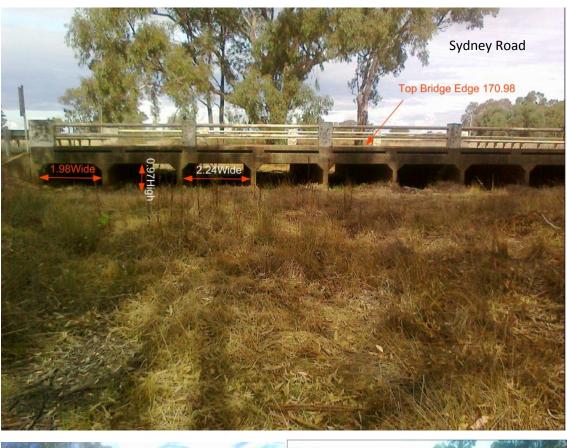




Figure 3-3 Examples of feature Survey by EDM group

Topography

The most significant data gap in this study was the lack of accurate topographical data within about 50% of the study area. Initially this data gap was attempted to be resolved with land based feature survey, but it quickly become evident that it could not be collected at sufficient resolution to accurately flood model the Kennedy Creek floodplain.

A sub-consultant was engaged to collect Unmanned Aerial Vehicle (UAV) topographic and photographic data. Australian UAV undertook several days of data collection flights in January 2015 to infill the region inside the study area with no high resolution topography. Interesting during this period a pipe laying contractor was laying the new gas main through the Kennedy Creek floodplain. This anomaly had to be later edited out of the topographic data.



The UAV topography was integrated with the available LiDAR topography to create a seamless topographic dataset across the study area. Some discontinuities were noted at the dataset boundaries. These were resolved by interpolating the two datasets at the boundaries to ensure water moved freely on the floodplain.



4. HYDROLOGICAL MODELLING

4.1 Overview

A RORB hydrological model was created to determine the 5 and 100 year ARI flow hydrographs affecting the study area. Catchment delineation of the upstream catchment was undertaken utilising the ArcHydro software package. The ArcHydro program employed the VicMap 10m DEM to determine watersheds throughout the catchment, from which the RORB model reach and sub-area data was derived. The RORB model layout is shown in

Figure 4-1.

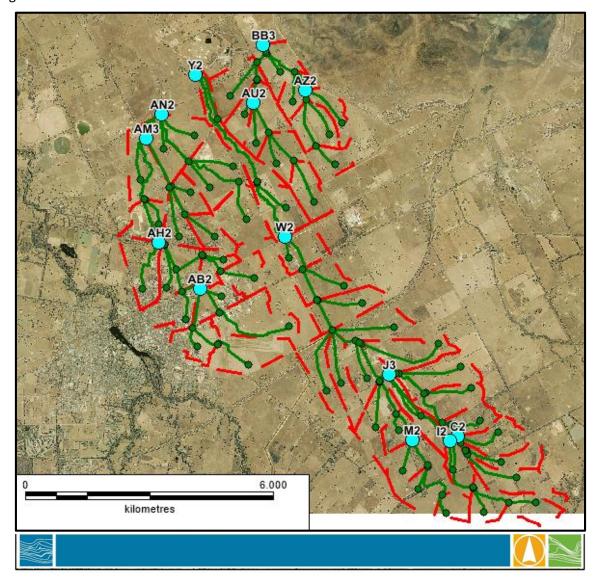


Figure 4-1 RORB model layout

Fraction imperviousness values for the RORB model sub-areas were assigned in accordance with the current planning scheme zoning layers, with each zoning layer allocated a fraction imperviousness value in accordance with Melbourne Water's current MUSIC Guidelines (MWC,2010). The fraction impervious values were then reviewed and adjusted if necessary following a general review of aerial photography and on site observations from a site visit conducted in December 2013.



Thirteen reconciliation points were identified in the setup of the hydrological model. The location of these inflow points is shown in Figure 4-3. The RORB model was reconciled against Rational Method estimates at these locations. Assumptions made in the Rational Method estimate included the following:

- Where possible assumptions and findings from the Enterprise Park assessment were adopted in this study. This included the IFD parameters applied;
- The weighted average fraction imperviousness (FI) values were determine using consistent values from the RORB model;

The resultant 100 year ARI Rational Method calculations for each inflow points is shown in Table 4-1.

Table 4-1 100 Year Rational Method Calculations

Sub-Catchment	Area	Weighted	Q100yr (m³/s)
Name	(Km²)	Average FI	Rational Method
C2	2.833	0.05	9.88
M2	2.563	0.05	9.19
12	2.131	0.05	8.04
J3	13.000	0.05	29.64
W2	21.583	0.061	44.11
Y2*	23.787	0.060	47.14
AB2	4.511	0.266	23.42
AH2	8.912	0.252	37.23
AM3	12.917	0.233	46.8
AN2*	17.518	0.05	59.86
AU2	2.638	0.05	9.39
AZ2	2.224	0.058	8.5
BB3*	8.139	0.056	21.59

^{*} Drainage line outlet

4.1.1 RORB Parameters

The RORB model was run with AR&R 1987 method with an aerial reduction factor area of 0.0 km². The Initial Loss applied was 15mm. Temporal patterns were fully filtered. Runoff coefficients used are shown in Table 4-2.

Table 4-2 Runoff Coefficient for ARI Events

ARI Event	Runoff Coefficient
5 Year	0.25
100 Year	0.60

RORB parameter files were created and made available with the RORB model to BRCC.

4.2 RORB Model Reconciliation

The RORB models were reconciled to match the 100 year ARI peak flow estimates from the Rational Method calculations. This was achieved by varying the kc parameter. Kc parameters are shown in Table 4-3.

Figure 4-2 shows hydrographs from the RORB model for the 100 year ARI 2 hour event (broadly the critical duration throughout the system).



Table 4-3 RORB model reconciliation and parameters

RORB detail		Q100yr (m3/s) Rational	Q100yr (m3/s) RORB	Кс	100yr Critical Duration	
C2	Hydr0001	9.88	9.86	1.80	2h	
M2	Hyd0002	9.19	9.19	2.22	2h	
12	Hyd0003	8.04	8.02	1.67	1h	
J3	Hyd0004	29.64	29.64	3.30	3h	
W2	Hyd0009	44.11	44.11	2.36	2h	
Y2*	Hyd0012	47.14	47.13	0.95	2h	
AB2	Hyd0017	23.42	23.38	1.93	1h	
AH2	Hyd0020	37.23	37.25	2.12	2h	
AM3	Hyd0025	46.8	46.80	1.57	2h	
AN2*	Hyd0031	59.86	59.86	2.24	2h	
AU2	Hyd0034	9.39	9.38	1.54	2h	
AZ2	Hyd0037	8.5	8.52	1.50	1h	
BB3*	Hyd0038	21.59	21.59	3.11	3h	

^{*} Drainage line outlet

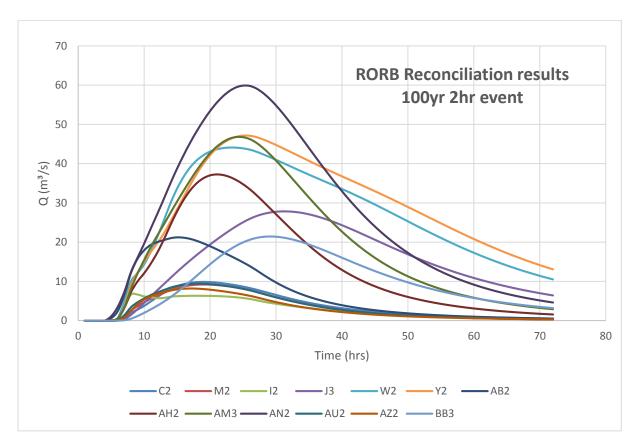


Figure 4-2 100 year ARI 2hr duration RORB Reconciliation results



4.3 RORB Model Inflows

The reconciled RORB model was then used to extract flows for the hydraulic model. 10 locations inflow were identified they are shown in Figure 4-3. Figure 4-4 and Figure 4-5 show inflow hydrographs applied in the 100yr ARI 2hr modelling which was broadly the critical duration throughout the system for that ARI. Table 4-4 shows representative peak inflows into the hydraulic modelling.

Table 4-4 RORB model peak flow results for 100yr 2hr & 5yr 6hr ARI events

RORB detail		Q100yr 2hr RORB Peak flow (m3/s)	Q 5yr 6hr RORB Peak flow (m3/s)	
R2*	Hyd0005	37.0	6.9	
U1	Hyd0006	6.7	1.0	
V1	Hyd0007	5.8	0.9	
W1	Hyd0008	8.9	1.1	
X1	Hyd0010	10.7	1.2	
AO1	Hyd0026	6.2	1.0	
AT2	Hyd0032	6.9	1.1	
Y1	Hyd0011	7.7	1.0	
AW1	Hyd0036	2.9	0.5	
AV1	Hyd0035	2.0	0.3	

^{*} Represented as the "external catchment" in Figure 4-5



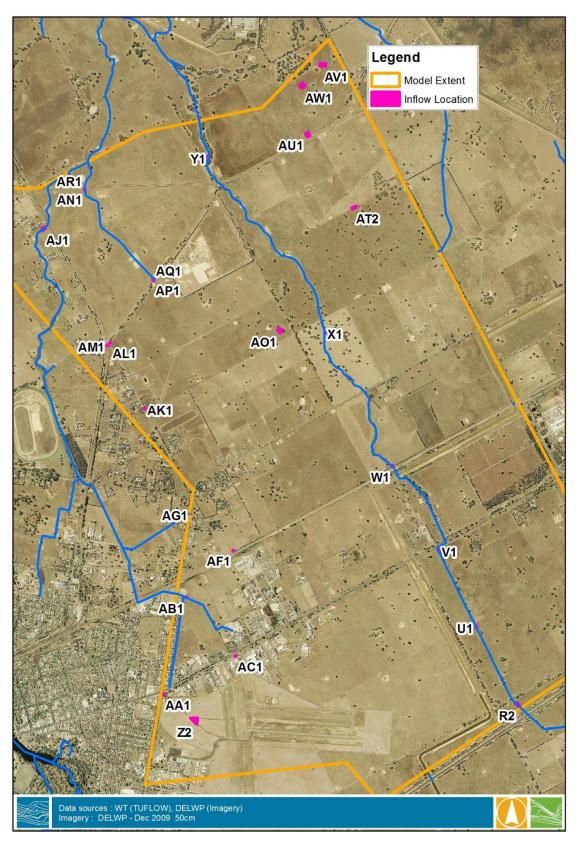


Figure 4-3 Inflow Locations



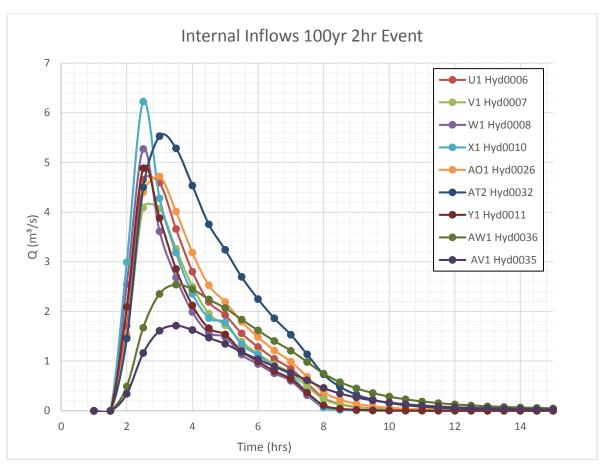


Figure 4-4 100 year ARI 2hr duration RORB Inflow Hydrographs (Internal flows)

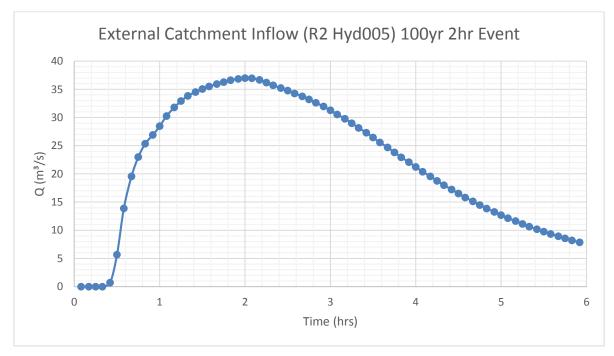


Figure 4-5 100 year ARI 2hr duration RORB Inflow Hydrographs (External Catchment flow)

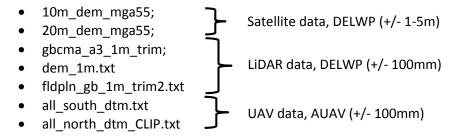


5. FLOOD MODELLING

5.1 Hydraulic Model

A detailed linked one-dimensional (1D) / two-dimensional (2D) TUFLOW¹ model has been created to simulate existing flood conditions. The model will allowed for the accurate representation of the overland flow paths throughout the study area.

The model topography was compiled using 7 individual datasets these included:



Preference was applied to LIDAR with this data supplemented by the UAV data collect by Australian UAV. Small areas not covered by either of these data sets were supplemented by satellite data. A review of the first pass model runs showed the areas represented by satellite data did not get wet and therefor didn't add any error to the overall results. A grid resolution of 4 m² was adopted to ensure adequate detail of the topography could be described. At the data boundaries (e.g. between LiDAR and the UAV datasets) some small discontinuities were observed. Where this was observed, the two datasets were smoothed out (interpolated) to insure overland flow was able to be accurately represented.

Manning's roughness values were adopted from Melbourne Water's Flood Mapping and Mitigation Technical Specifications and Requirements document. 'HQ' boundaries were used to convey overland flow out of the catchment in a steady manner.

1D networks such as underground pipes, and major crossings were incorporated in the model build. Pipes were modelled as 1D links using the MapInfo dataset provided by BBRC. This data was heavily supplemented by survey from EDM Group. The key waterway crossings were modelled using the structure details provided by EDM Group.

100 and 5 year ARI overland flow hydrographs determined though the RORB modelling mentioned within this report were input into the TUFLOW model at the ten inflow locations mentioned previously. Hydrology from the events shown in Table 5-1 were run in the TUFLOW under existing and mitigated conditions. The TUFLOW model setup is shown in Figure 5-1.

Table 5-1 Events and scenarios modelled in this study

Condition	ARI	1hr	2hr	4.5hr	6hr	12hr
Existing Conditions	100	✓	√	✓	√	✓
	5	√	√	√	✓	✓
Mitigated conditions	100	✓	✓	✓	√	✓

The results presented in this report are the worst flooding seen in all the conditions modelled (results have been enveloped together).

¹ TUFLOW is a standard hydrodynamic modelling package used extensively by Melbourne Water to undertake urban flood investigations.

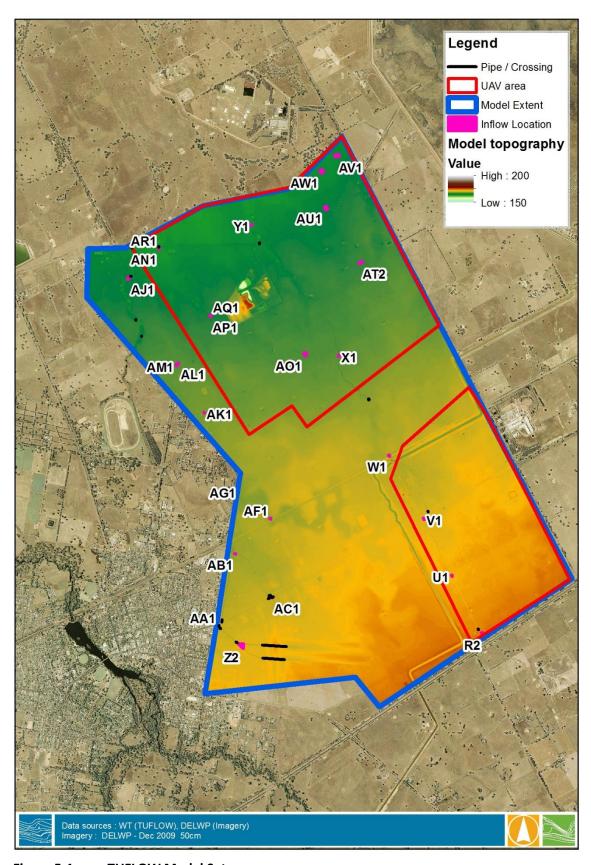


Figure 5-1 TUFLOW Model Setup



6. EXISTING CONDITIONS FLOOD MODELLING RESULTS

6.1 Overview

Flood modelling results were produced from south of the Hume highway through to Link Road about 5 kilometres north east of Benalla's CBD. The main focus of the investigation was the Kennedy Creek floodplain. The northern section of the Eastern Main drain system was included in the model to analyse if any interaction between the systems occurred. Broadly speaking, they were shown to be independent. Flood modelling results from northern sections of the Eastern Main Drain have been covered in the study maps and should not be used to estimate flooding in that section of the system. Preliminary feedback from BRCC Engineers suggested the flood model would need to be extended north to accurately represent flooding in this area.

100 year and 5 year ARI flood extents are shown in Figure 6-1. Both flood extents are quite similar reflecting the flat nature of the floodplain. Between the Hume highway and the railway line, flow paths are wide and shallow (200-400 mm deep). Around the northern boundary of this area, the flow path is significantly constricted as it flows past the Mokoan Channel syphon feature. Water tends to back up in this area, with the railway effectively acting as a defacto retarding basin, stopping some flood water from moving north.

Flood water is shown to impact Sydney Road in both 100 year and 5 year ARI event. This is expected as advice from VicRoads at the beginning of the project suggested the road was regularly cut by flood water. Historic flood levels from the VFD were compared to 100 year ARI levels. The two recorded levels between the Hume Highway and Sydney road were reasonably accurate sitting

North of the railway line some flood water flows west towards town stopping at the western boundary of the current industrial estate. However, the majority of the flow moves north towards Murray's Road, again flood water tends to back up behind the raised road feature. Flood depths on the southern side of Murrays road are deep, >1 m. Flood levels and extents in this area were noted to be "highly accurate" by local resident and farmer Ken Jaldous. Ken lived on the Kennedy Creek floodplain during the devastating 1993 flood event and suggested the 100 year ARI mapping was millimetre perfect at his property when compared to that event.

North of Murrays Road to a point level with the Reuse Depot (off Old Farley Road) flow on the floodplain is again broad and shallow (100-300 mm). North of the Reuse depot, flow paths become more constricted and deeper, before backing up behind Link Road. This area marks the northern boundary of the flood mapping project.

Flood maps for the 100 year ARI event for following parameters (show below) are shown on the following pages. Results presented equate to the worst flooding observed in the 1, 2, 4.5, 6 and 12 hour duration 100 year ARI events. Maximum results from each of these modelling runs were enveloped to produce the following maps:

- Peak Flood Depth (m) Figure 6-2;
- Peak Water Surface Elevation (m AHD) Figure 6-3;
 - These results could be thought of as the "flood levels" determined in the study;
- Peak Flood Velocities (m/s) Figure 6-4; and
- Flood Hazard (i.e. the greater of Velocity x Depth (m²/s) or depth (m)) Figure 6-5;
 - Further discussion of flood hazard criteria can be found in the Enterprise Park drainage strategy.

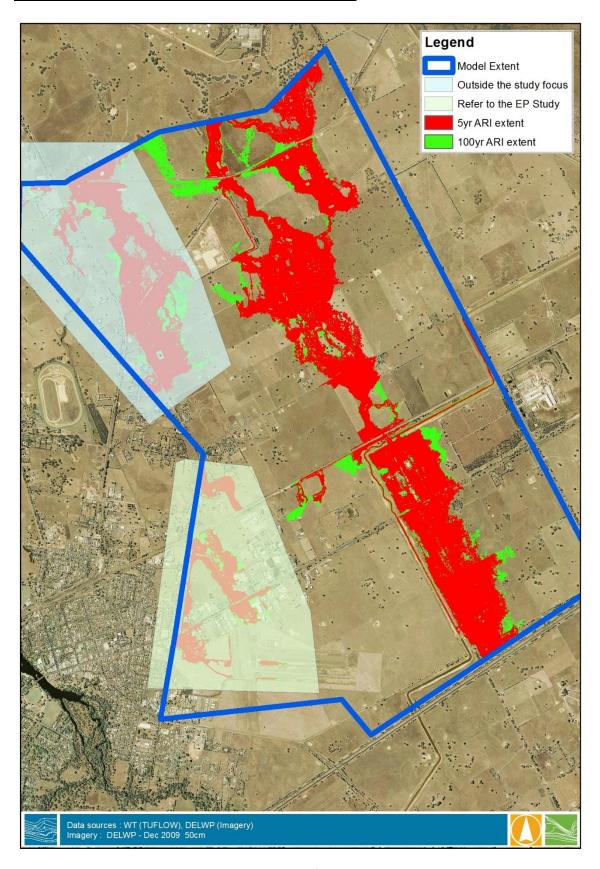


Figure 6-1 Flood Extents 5 and 100 year ARI flood extents – Study Area



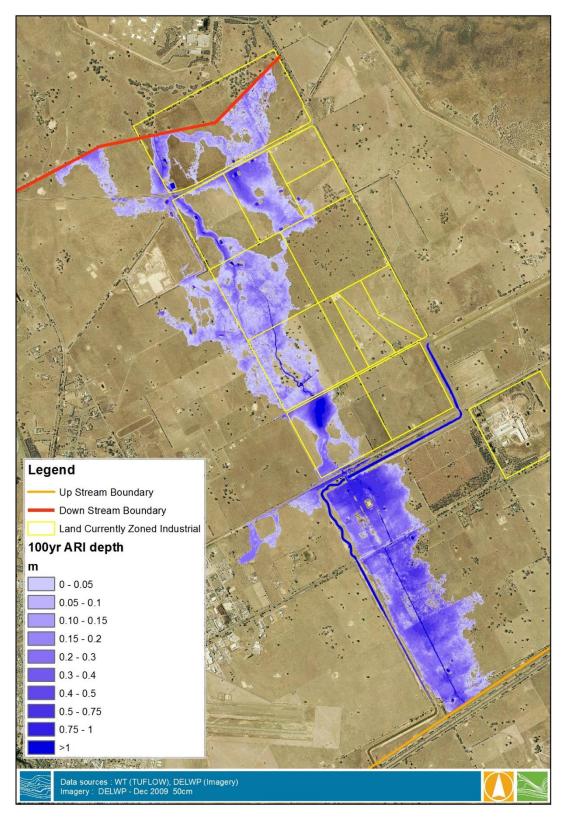


Figure 6-2 Flood Depth 100 year ARI event—Study Area



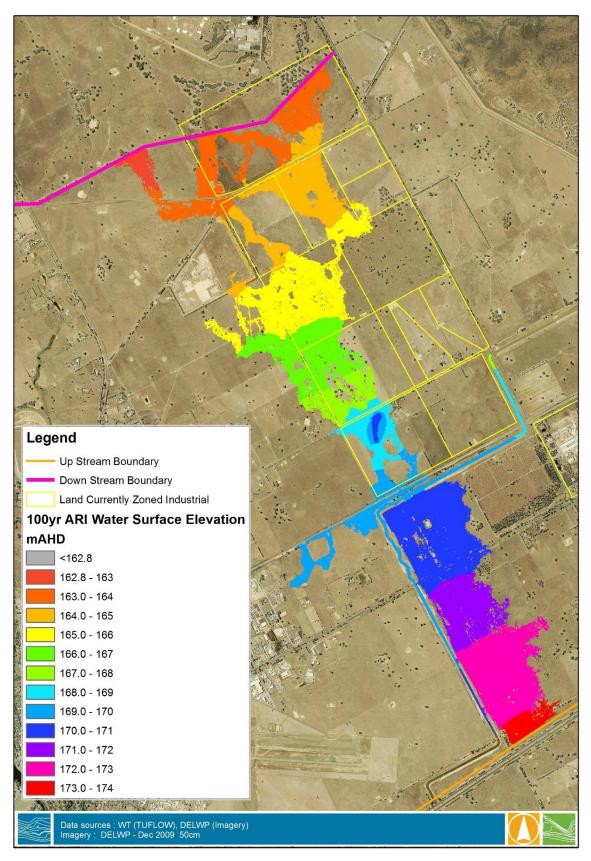


Figure 6-3 Flood Levels (Water Surface Elevation) 100 year ARI event—Study Area



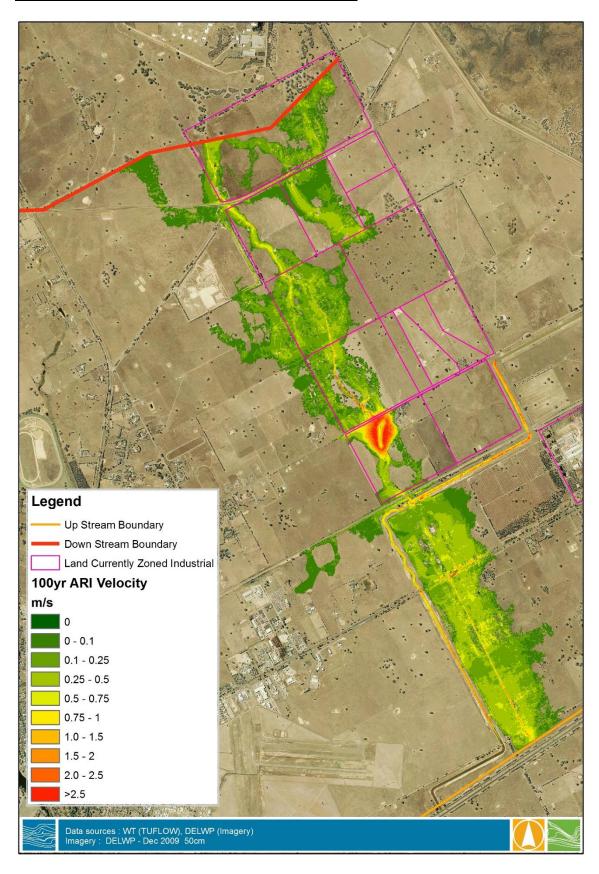


Figure 6-4 Flood Velocity 100 year ARI event—Study Area



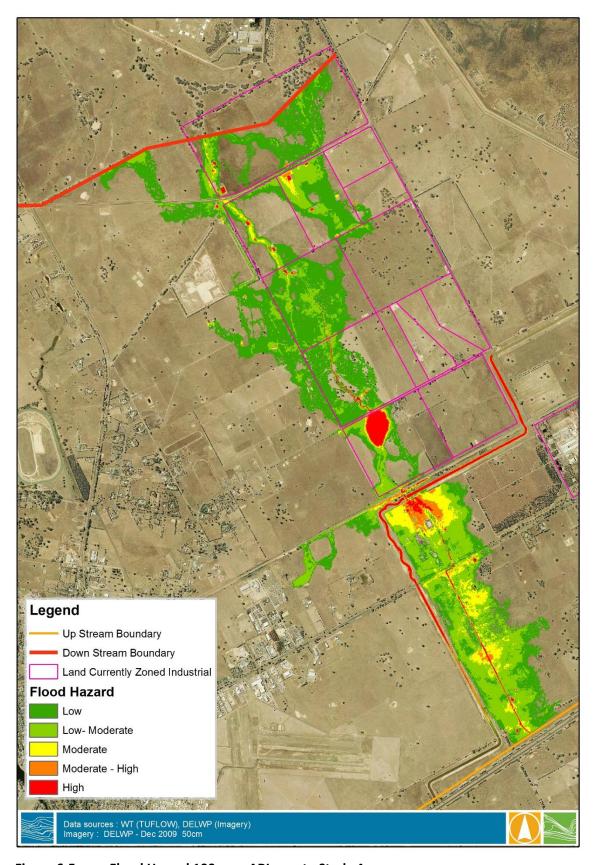


Figure 6-5 Flood Hazard 100 year ARI event— Study Area



6.2 Result Checks

Flood modelling results were provided to VicRoads, Goulbourn Broken CMA, BRCC staff and community representatives for comment. All stakeholders who responded suggested the mapping was accurate.

Guy Tierney of the GBCMA asked to have the results checked against the 1993 spot levels in the Victorian Flood database. The 1993 flood event has been estimated by others as equivalent to a 1 in 100 year ARI event. 6 levels exist within the study area.

At the upstream end of the study area (between the Hume freeway and Sydney road) flood levels were about 200 mm higher than the 1993 flood levels. While between Sydney Road and the railway crossing results were about 500 mm lower than 1993 flood levels. North of the railway line at Murray's Road the hydraulic model results did not extend to the recorded flood level (boundary of the flood extent was 8 m away), Further north adjacent to the refuse depot the hydraulic models flood level was within 200 mm of that recorded 1993.

Overall given the high level nature of the investigation the flood modelling results are considered fit for purpose

6.3 100 year ARI results within the land zoned industrial

The major diver of this study was to get a better understanding of flooding within the land North East of the Benalla which is currently zoned for industrial land use but is currently used for primary production (farming).

The flood modelling results showed approximately half of this area is subject to flooding from the Kennedy Creek. It is noted that without the railway line and the Mokoan Channel syphon feature, flooding of this land would likely be more significant. While the extent of flooding is significant, flood depths and velocities are not high, suggesting fill could be placed in the floodplain without significant impacts.

It is envisaged that while flooding would place some constraints of development inside this area, however, mitigation options may be available to allow some land to be developed. Any development would be subject to controls set by the GBCMA and would require further modelling to be realised.



7. MITIGATED CONDITIONS FLOOD MODELLING

7.1 Overview

As part of the project scope, Water Technology undertook a desktop analysis of available mitigation options for the Kennedy Creek floodplain. Mitigation was focused on;

- Relieving existing flooding at Sydney Road; and,
- Reducing flooding inside the land proposed for industrial development.

Each of the options considered are discussed below. These options were presented to the project steering committee November 2014. Advice was provided to Water Technology to further progress the option of a diversion into Kennedy Creek in February 2015.

7.1.1 Retarding basin

Constructing a retarding basin upstream of Sydney Road off-line from Kennedy Creek, and incorporate a high-flow diversion from the creek to the basin was considered as one option to reduce flooding. This option would result in a slow discharge outlet to the Creek at a controlled rate so as not to exceed the capacity of the primary creek channel.

There are a range of locations available upstream of Sydney Road with potentially suitable land, however there are two main points to consider regarding the location:

- 1. The closer to Sydney Road the basin is sighted, the more catchment it can retard, and therefore reduce not only flooding to Sydney Road, but also the flood overlay downstream within potential industrial land, thereby improving the value of said land;
- 2. The closer to the Hume Freeway that the basin is constructed, more land between Hume Freeway and Sydney Road may be able to be developed as industrial land as the flood overlay could be reduced in this area by having the retarding basin upstream of it.

Figure 7-1 shows two potential locations for basin features. These considerations would not only impact the location of the basin, but also the size of the basin, as a greater catchment will require a larger basin.

Design considerations included:

- Proximity to Sydney Road
 - The closer the basin is to Sydney Road, the easier it will be to use the existing culverts under Sydney Road for Kennedy Creek as a controlled discharge point for the basin – saving on cost and time during construction.
- Water Re-use;
 - Water collected in the retarding basin could be harvested and reused within the industrial estate e.g. as construction water during development or for plant operations, etc.

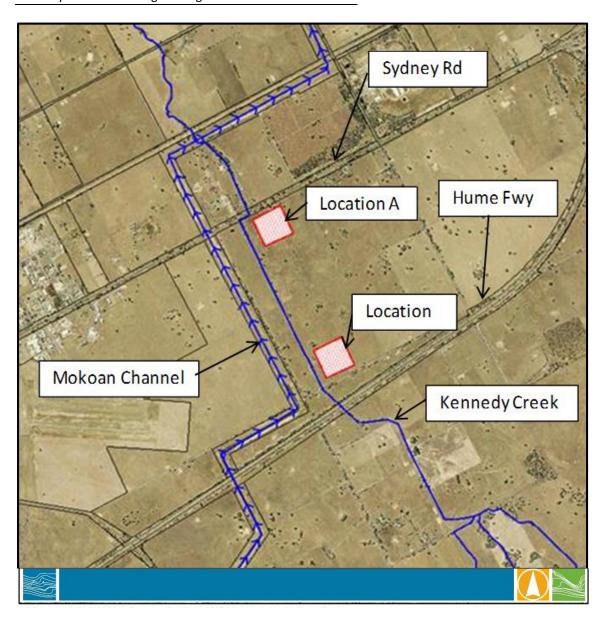


Figure 7-1 Mitigation option analysis – Retarding Basins

7.1.2 Mokoan Channel

The Mokoan Channel could be used for 3 purposes, if Kennedy Creek is diverted to the channel upstream of Sydney Road;

- 1. Conveyance channel for high flows from Kennedy Creek to the Winton Wetlands;
- 2. Retarding basin for high flows from Kennedy Creek, with the flows discharged back to Kennedy Creek just south of the rail line at a controlled rate; or,
- 3. A combination of the above.

The intent with this option was to allow a continuous low flow through the channel from the retarding area to the wetlands, so as not to disrupt the existing function of the wetlands, and only discharge high flows back into Kennedy Creek as required. Figure 7-2 shows concept for the retarding basin within the channel.



A Manning's calculation of the channel capacity indicates it is in the order of 120 m³/s, with the flows upstream of Kennedy Creek in the order of 40 m³/s, suggesting there could be capacity for the Kennedy Creek flows within the Mokoan Channel. One potential challenge in this option is the capacity of the syphon feature near the railway crossing were the Mokoan Channel turns from flowing north to east crossing the Kennedy creek floodplain. No structure details were collected by EDM group during their feature survey.

Ultimately this mitigation option would need to demonstrate that:

- The rail line is unaffected by the proposed works;
- The channel itself would not flood Sydney Road;
- The drainage of the natural catchment of the channel will not be affected by this proposal;
- The weir level set (in the instance of a retarding basin) does not cause flooding/overtopping of the channel banks upstream; and,
- The diverted flow was accepted by the Winton Wetlands committee of management.

Many options exist to achieve this design criteria, however Water Technology modelling only looked at proving the concept was viable, with the assumption that further work would be required to refine any option trialled.

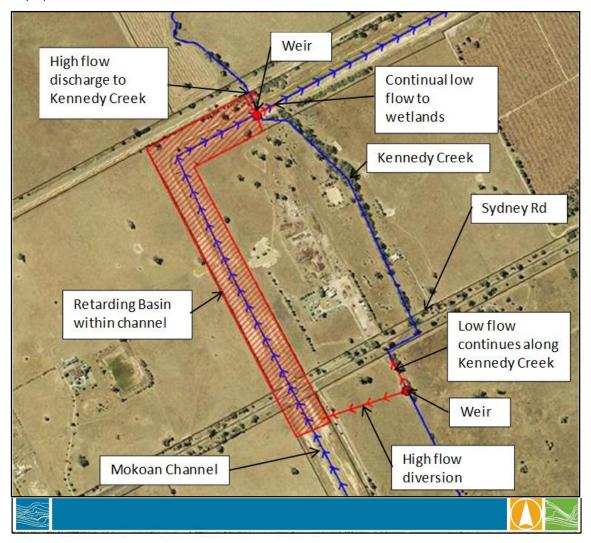


Figure 7-2 Mitigation option analysis – Diversion to Kennedy Creek



7.1.3 Alter Sydney Road/ and/or the culverts under it

The final option is to either raise and/or increase the capacity of the culverts under Sydney Road to allow Kennedy Creek to drain more freely. This could mitigate the flooding on Sydney Road, but would do little to reduce the flood overlay on the industrial land downstream of Kennedy Creek. It would also likely be the most expensive option, disrupt traffic flows along Sydney Road, and there may not be sufficient space within the road reserve to increase the level of the road. For these reasons, it was the least desirable option.

7.2 Preliminary Mitigation modelling results

7.2.1 Discussion

As discussed it was resolved by the steering committee to further investigate the diversion to the Mokoan channel mitigation option. Initial mitigation modelling runs trailed focused on diverting flood water through the southern road reserve along Sydney Road. Unfortunately the reserve area wasn't wide enough to achieve significant reductions in flooding at Sydney Road. The next iteration of the modelling process looked at using the Hume Freeway road reserve, again the option could not be made to work without using some privately owned land.

The mitigation modelling results adopted in this report involve acquiring about 13 ha of land immediately north of the Hume Freeway. Under this scenario, this land would remain available to the land owner for grazing. Equally the same arrangement could be implemented closer to Sydney Road.

The works shown in Figure 7-3 were tested in the hydraulic model. They included modest bunding (\sim 500 mm high), a diversion channel (15 m wide) and a low flow pipe arrangement through the bunding ($3x\Phi$ 900 mm RCPs). Using this treatment resulted in around 90% of the external catchment flows being diverted into the Mokoan Channel.

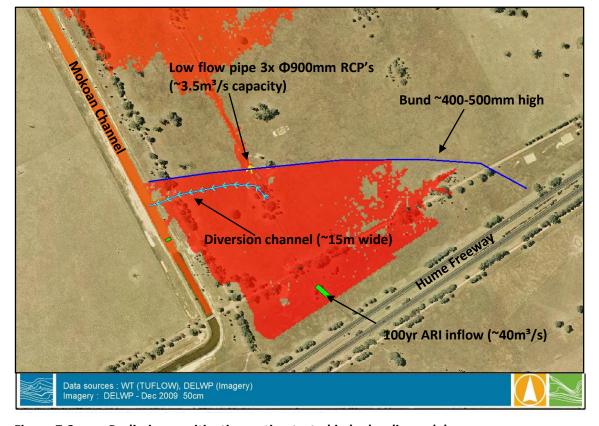


Figure 7-3 Preliminary mitigation option tested in hydraulic model



Adopting the aforementioned conditions resulted in significantly less flooding between the Hume Freeway and the railway crossing (as shown in Figure 7-4). Unfortunately it did not significantly reduce flooding north of the railway line within the industrial zoned land. Flooding of Sydney Road is managed with this solution with all flooding of the road practically eliminated.

Figure 7-5 shows the change in flood levels between the Hume freeway and the Railway line. Regions shaded in green show area where flood levels have been reduced. Yellow and orange regions are where flood levels have increased. Through the area shown in Figure 7-5, flood levels are largely reduced by 200 mm+. The exception to this is behind the bunded area and inside the Mokoan Channel.



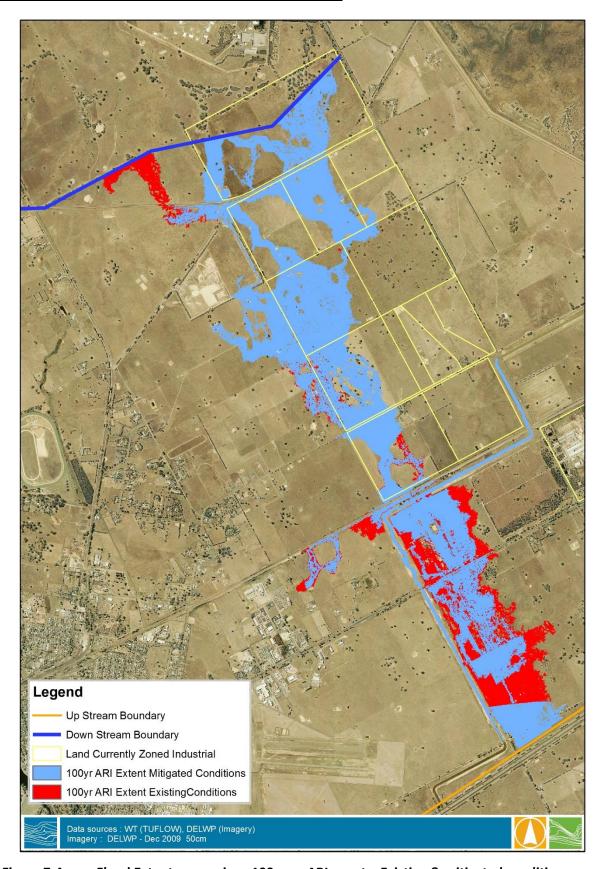


Figure 7-4 Flood Extent comparison 100 year ARI event – Existing & mitigated conditions



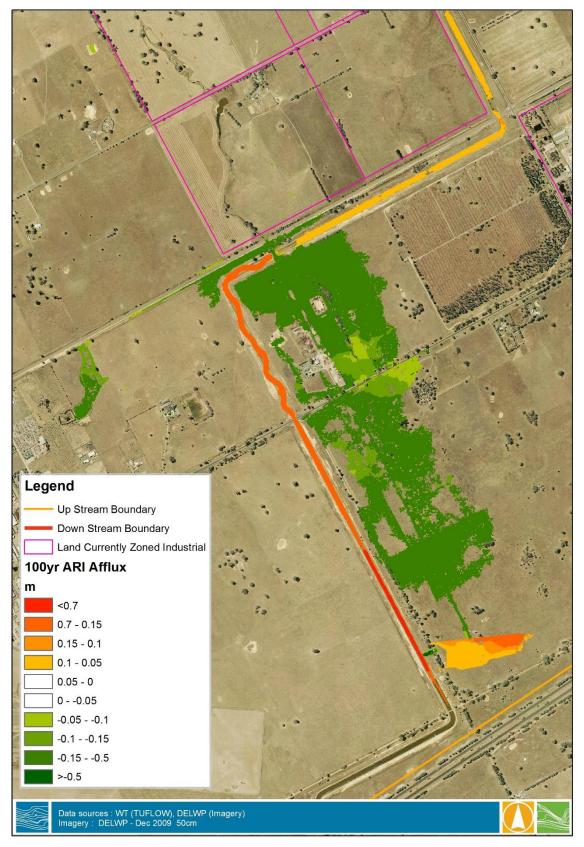


Figure 7-5 Flood Level difference plot (Afflux) - 100 year ARI event—Study Area



8. SUMMARY & CONCLUSION

Hydrologic and hydraulic analysis by Water Technology has been undertaken to describe the current flooding conditions on the Kennedy Creek floodplain from the Hume Freeway through to Link Road 5 km north of the Benalla CBD.

Flood modelling results were presented to various stakeholders for comment. Generally the flood mapping been accepted as an accurate description of flooding inside the study area. Flood maps produced showed:

- Broad shallow flooding between the Hume freeway and the railway line crossing;
- Breakout flows moving west towards the town from the railway / Mokoan channel syphon area;
- Deeper more concentrated flows between the railway line and Murray Road, with significant ponding behind Murray Road;
- Shallow broad flooding between Murray Road and the refuse depot; and,
- More concentrated flows from the reuse depot to the study boundary.

The major diver of this study was to get a better understanding of flooding within the land North East of the Benalla which is currently zoned for industrial land use but is currently used for primary production (farming).

The flood modelling results showed approximately half of this area is subject to flooding from the Kennedy Creek. Flood depths and velocities inside the region are not high suggesting fill could be placed in the floodplain without significant impacts. Flooding would place some constraints of development inside this area, but mitigation options may be available to allow land to be developed.

Several options were presented to BRCC to mitigate flooding on the Kennedy Creek floodplain, with options focused on reducing flooding in the land zoned industrial and stopping Sydney Road being inundated.

It was resolved to investigate diverting flood flows into to the Mokoan Channel as mitigation option. This modelling showed that significant reductions (200 mm+) in flood levels could be achieved between the Hume Freeway and the railway line. North of this area only small changes in the maximum flood extents were observed. This result reflected original conclusions that the Mokoan Channel syphon and the railway crossing provide significant attention for the land to the north already.



APPENDIX A RORB MODELLING DETAIL



Catchment File:

```
Kennedy Ck - 0
CRORB GE 6.15
C WARNING - DO NOT EDIT THIS FILE OUTSIDE RORB TO ENSURE BOTH GRAPHICAL AND CATCHMENT DATA ARE COMPATIBLE WITH EACH OTHER
C THIS FILE CANNOT BE OPENED IN EARLIER VERSIONS OF RORB GE - CURRENT VERSION IS v6.15
C Kennedy Ck - 0
C #FILE COMMENTS
C File created using MiRORB version 1.2
C Original CATG file created on 27/05/2014 at 15:41:32
C #SUB-AREA AREA COMMENTS
C Sub-area areas in km2
C#IMPERVIOUS FRACTION COMMENTS
C 0
C#BACKGROUND IMAGE
C T F M:\Jobs\3000-3099\3014_Enterprise_Park_Developme\Analysis\RORB_KennedyCk\KennedyCk\Ex.wmf
C#NODES
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                   14.391
                              1.000 1 0 55 A
                                                          1.179
                                                                    0.050 0 0
   2
         67.541
                    17.328
                              1.000 1 0 56 B
                                                          0.699
                                                                    0.050 0 0
   3
         67.130
                    22.805
                              1.000 1 0 59 C
                                                          0.955
                                                                    0.050 0 0
   4
                              1.000 1 0 61 D
                                                          0.683
                                                                    0.048 0 0
         65.189
                    24.967
С
   5
         64.141
                    31.017
                              1.000 1 0 64 E
                                                          1.028
                                                                    0.050 0 0
   6
         63.015
                    34.887
                              1.000 1 0 66 F
                                                          1.268
                                                                    0.050 0 0
   7
         71.345
                    9.867
                              1.000 1 0 68 G
                                                          0.782
                                                                   0.050 0 0
         65.577
                    8.049
                              1.000 1 0 69 H
                                                          0.694
                                                                    0.050 0 0
   9
                              1.000 1 0 71 1
         62.154
                    15.894
                                                         0.655
                                                                   0.050 0 0
   10
          58.682
                    25.850
                               1.000 1 0 73 J
                                                          0.804
                                                                    0.050 0 0
   11
          61.378
                    7.855
                              1.000 1 0 75 K
                                                          0.460
                                                                    0.050 0 0
   12
          58.351
                    12.247
                               1.000 1 0 76 L
                                                           1.057
                                                                    0.050 0 0
                               1.000 1 0 78 M
                                                                     0.050 0 0
   13
          56.593
                    15.622
                                                           1.046
С
   14
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                    23.085
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                                                           0.574
                                                                     0.050 0 0
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          53.590
                    23.638
                               1.000 1 0 82 0
                                                           1.116
                                                                     0.050 0 0
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          52.617
                    32.755
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                                                           0.988
                                                                     0.050 0 0
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                                                                     0.050 0 0
   18
                               1.000 1 0 89 R
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                    42.138
                                                           1.247
                                                                    0.129 0 0
   19
          49.809
                    31.828
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                                                           0.910
                                                                    0.075 0 0
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          48.197
                    30.461
                               1.000 1 0 92 T
                                                           1.061
                                                                    0.099 0 0
   21
                    46.552
                               1.000 1 0 93 U
                                                           1.195
                                                                     0.055 0 0
   22
                               1.000 1 0 95 V
                                                                    0.054 0 0
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                    49.888
                                                           1.063
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                    55.078
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                                                                     0.114 0 0
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                                                           0.961
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                    80.665
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                                                                     0.050 0 0
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                    35.170
                               1.000 1 0 103 Z
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   27
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                    38.659
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          44.020
                    42.356
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                                                            0.762
                                                                      0.434 0 0
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                                                            1.114
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          36.813
                    54.568
                               1.000 1 0 116 AG
                                                            0.514
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                    49.275
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С							
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c	37	35.842	58.866	1.000 1 0	124 AK	0.670	0.207 0 0
С							
C	38	31.908	58.866	1.000 1 0	127 AM	0.547	0.172 0 0
C C	39	33.160	67.002	1.000 1 0	125 ΔΙ	0.711	0.231 0 0
c	33	33.100	67.002	1.000 1 0	125 AL	0.711	0.231 0 0
C	40	30.151	75.040	1.000 1 0	129 AN	0.819	0.596 0 0
С							
c c	41	39.324	62.147	1.000 1 0	130 AO	1.261	0.095 0 0
c	42	35.297	67.968	1.000 1 0	131 AP	0.737	0.051 0 0
c							
C	43	37.788	71.817	1.000 1 0	133 AQ	0.949	0.193 0 0
C C	44	22 610	77 500	1.000 1 0	12E AD	0.035	0.599 0 0
c	44	33.618	77.598	1.000 1 0	135 AK	0.835	0.599 0 0
c	45	46.786	65.380	1.000 1 0	136 AS	0.836	0.050 0 0
С							
C C	46	42.681	72.478	1.000 1 0	137 AT	0.919	0.050 0 0
c	47	40.437	76.239	1.000 1 0	139 AU	0.883	0.050 0 0
č	.,	101 107	70.203	1.000 1 0	2037.0	0.000	0.000
С	48	39.094	81.458	1.000 1 0	141 AV	0.701	0.050 0 0
C	40	27.474	04.000	1 000 1 0	442 414	4 000	0.050.0.0
c c	49	37.471	84.899	1.000 1 0	143 AW	1.082	0.050 0 0
c	50	49.150	72.263	1.000 1 0	145 AX	0.643	0.076 0 0
c							
С	51	48.478	77.714	1.000 1 0	146 AY	0.977	0.052 0 0
C C	52	49.860	79.855	1.000 1 0	1/19 //7	0.604	0.050 0 0
c	J_	43.000	75.055	1.000 1 0	140 AL	0.004	0.050 0 0
С	53	44.422	83.848	1.000 1 0	150 BA	0.613	0.064 0 0
C			00.05=		450.00		
c c	54	43.077	89.267	1.000 1 0	152 BB	0.881	0.078 0 0
c	55	63.740	18.777	1.000 0 0	57 A1	0.000	0.000 0 0
С							
С	56	63.613	19.402	1.000 0 0	57 B1	0.000	0.000 0 0
C C	57	63.511	19.384	1.000 0 0	58 B2	0.000	0.000 0 0
c	٠,	03.311	13.304	1.000 0 0	30 02	0.000	0.000 0 0
С	58	63.017	20.886	1.000 0 0	60 B3	0.000	0.000 0 0
С							
C C	59	62.709	21.993	1.000 0 0	60 C1	0.000	0.000 0 0
c	60	62.649	22.026	1.000 0 0	62 C2	0.000	0.000 72 0
C K	ennedy	/CkEUpper_	C2				
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C							
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С							
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c c	67	55.905	33.719	1.000 0 0	84 F2	0.000	0.000 0 0
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C C	69	64.410	12.510	1.000 0 0	70 H1	0.000	0.000 0 0
c	70	64.375	12.744	1.000 0 0	72 H2	0.000	0.000 0 0
C							
С	71	61.901	21.082	1.000 0 0	72 I1	0.000	0.000 0 0
c c	72	61.879	21.224	1.000 0 0	74 12	0.000	0.000 72 0
		/CkMUpper		1.000 0 0	7412	0.000	0.000 72 0
С	73	55.125	33.208	1.000 0 0	74 J1	0.000	0.000 0 0
C		FF 405	22 225	4 000 - 1	04.13	0.000	0.000 0 -
c c	74	55.095	33.283	1.000 0 0	84 J2	0.000	0.000 0 0
c	75	61.032	11.202	1.000 0 0	77 K1	0.000	0.000 0 0
c							
C	76	59.350	16.464	1.000 0 0	77 L1	0.000	0.000 0 0
C C	77	59.360	16.682	1.000 0 0	79 L2	0.000	0.000 0 0
c	,,	39.300	10.002	1.000 0 0	, , , , ,	3.000	J.000 U U
С	78	57.584	21.279	1.000 0 0	79 M1	0.000	0.000 0 0
C	70	F7 645	24 225	4 600 - 1	04.842	0.00-	0.000 == 1
C K	79 Sennedy	57.647 CkWUpper	21.396 M2	1.000 0 0	81 M2	0.000	0.000 72 0
	cu\	-curropher					
С	80	55.857	26.109	1.000 0 0	81 N1	0.000	0.000 0 0
С	80						
		55.857 55.851	26.109 26.218	1.000 0 0		0.000	0.000 0 0



C C	82	54.041	32.104	1.000 0 0	83 O1	0.000	0.000 0 0
c	83	54.051	32.176	1.000 0 0	74 02	0.000	0.000 0 0
C C	84	55.075	33.407	1.000 0 0	85 J3	0.000	0.000 72 0
		CkHilbrickR	_	1 000 0 0	87 J4	0.000	0.000 0.0
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C C	86	51.410	39.617	1.000 0 0	87 P1	0.000	0.000 0 0
С	87	51.405	39.674	1.000 0 0	90 P2	0.000	0.000 0 0
c c	88	52.012	39.217	1.000 0 0	85 Q1	0.000	0.000 0 0
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c c	93	47.187	47.198	1.000 0 0	94 U1	0.000	0.000 70 0
c u c	1 94	47.155	47.219	1.000 0 0	96 U2	0.000	0.000 0 0
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c c v		45.503	52.856	1.000 0 0	96 V1	0.000	0.000 70 0
C C	96	45.483	52.863	1.000 0 0	98 V2	0.000	0.000 0 0
C C V	97	43.551	58.845	1.000 0 0	98 W1	0.000	0.000 70 0
C	98	43.549	58.859	1.000 0 0	100 W2	0.000	0.000 72 0
C K	ennedy 99	CkRailwayL 40.469	ine_W2 68.930	1.000 0 0	100 X1	0.000	0.000 70 0
c x		40.457	68.956	1.000 0 0		0.000	0.000 0 0
С							
C CY	101 1	33.859	88.790	1.000 0 0	102 Y1	0.000	0.000 70 0
C	102	33.702	88.870	1.000 0 0	155 Y2	0.000	0.000 72 0
C	103	CkOutlfall_ 36.197	39.090	1.000 0 0	104 Z1	0.000	0.000 0 0
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CA	A1						
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C B	enellaR 109	ailwayLine_ 36.390	_AB2 44.919	1.000 0 0	108 AC1	0.000	0.000 70 0
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С							
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c c	112	31.490	52.918	1.000 0 0	114 AD3	0.000	0.000 0 0
С	113	30.222	57.334	1.000 0 0	114 AE1	0.000	0.000 0 0
	114	30.209	57.410	1.000 0 0	119 AE2	0.000	0.000 0 0
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C A C	F1 116	34.546	55.419	1.000 0 0	117 AG1	0.000	0.000 70 0
C A	G1						
c c	117	34.448	55.450	1.000 0 0	112 AG2	0.000	0.000 0 0
c c	118	29.363	57.845	1.000 0 0	119 AH1	0.000	0.000 0 0
c	119	29.651	57.899	1.000 0 0	121 AH2	0.000	0.000 72 0
	enellak 120	acecourseR 28.083	65.693	1.000 0 0	121 AI1	0.000	0.000 0 0
C C	121	28.060	65.759	1.000 0 0	123 AI2	0.000	0.000 0 0
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c c	123	28.119	76.776	1.000 0 0	128 AJ2	0.000	0.000 0 0
C	124	32.538	63.037	1.000 0 0	126 AK1	0.000	0.000 70 0
	125	30.959	68.067	1.000 0 0	126 AL1	0.000	0.000 70 0
C A C	L1 126	30.820	67.988	1.000 0 0	128 AM2	0.000	0.000 0 0
С							
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C C B	128 enellaN	28.186 IorthRd_AN	76.992 13	1.000 0 0	134 AM3	0.000	0.000 72 0



c :	129	29.908	81.427	1.000 0 0	134 AN1		0.000	0.000 70 0			
C Al	V1	38.494	69.008		132 AO1		0.000				
C AC		32.955	72.733	1.000 0 0	132 AP1		0.000	0.000 70 0			
C AF	21	32.935	72.777		134 AP2		0.000				
C :		32.973	72.779		132 AQ1		0.000				
CAC	21										
	enellaOı	29.948 itlet_AN2	81.506		155 AN2		0.000				
C AF	R1	29.987	81.377				0.000				
C :	136	44.611	72.894	1.000 0 0	138 AS1		0.000	0.000 0 0			
C :	137	41.799	78.045	1.000 0 0	138 AT1		0.000	0.000 0 0			
C 2		41.805	78.174	1.000 0 0	140 AT2		0.000	0.000 70 0			
C AL	139	40.100	83.483	1.000 0 0	140 AU1		0.000	0.000 70 0			
C :	140	40.101 nkRdWest_	83.599	1.000 0 0	142 AU2		0.000	0.000 72 0			
C :	141	40.480	87.720	1.000 0 0	142 AV1		0.000	0.000 70 0			
C A\		40.513	87.868	1.000 0 0	144 AV2		0.000	0.000 0 0			
C 1	143	40.486	91.011	1.000 0 0	144 AW1		0.000	0.000 70 0			
C A\		40.532	91.074	1.000 0 0	153 AW2		0.000	0.000 0 0			
C :	145	47.003	75.562	1.000 0 0	147 AX1		0.000	0.000 0 0			
C :	146	45.920	84.186	1.000 0 0	147 AY1		0.000	0.000 0 0			
C C		45.908	84.213				0.000				
C :		45.829	85.899				0.000				
C 1		45.792	85.924				0.000	0.000 72 0			
C Ea	stTribLi	nkRdEast_A	AZ2								
C 1		44.700	89.304				0.000				
C 1		44.705	89.341				0.000				
C :	152	41.555	92.684	1.000 0 0	153 BB1		0.000	0.000 0 0			
C 1	153	41.497	92.975	1.000 0 0	154 BB2		0.000	0.000 0 0			
C 1		41.190 utlet_BB3	94.278	1.000 0 0	155 BB3		0.000	0.000 72 0			
C :	155	29.779	95.308	1.000 0 1	0		0.000	0.000 0 0			
С	REACHES	:									
C :			1 55	010	1.402	1.358	2.0				
С	68.38	39 65.7		64.130	1.402	1.556	30				
	14.48 2 A1-B2	2	54 : 55 57		0.095	0.483	1 0				
С	63.62 19.08	34									
	66.50	04 65.9	58 (0 1 0 65.335 65.1	169 64	.889	64.700	64.591 64.054	63.942	63.829	63.613
c c				18.222 18.3 0 5 0				18.983 19.010	19.118	19.433	19.402
c c	63.50 19.39										
С	5 B2-B3	3	57 58	010	0.224	0.403	1 0				
C	20.13	35		010	1 166	1 760					
c	66.84	1 66.6	76 (66.511 65.7	722 64	.692	64.298				
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С	62.6	79									
c	22.00 8 B3-C2	2	58 60	010	0.170	0.473	1 0				
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	9 C2-D2	39	60 62	010	0.332	0.495	1 0				
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С	64.99	97 64.6	05 (64.181 63.5	16 62	.868	- 0				
c c				24.163 23.9 0 5 0			1 0				
С	61.96 24.23	53									
С	12 D2-D	3		010	0.225	0.677	3 0				
		30 61.4 12 24.2									
				020	0.942	0.318	10 0				



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C 27 J-J1
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c	26.254	26.934 27.7				30.274					31.290				
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c	51.790														
С	36.050														
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С	39.645														
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c	41.650														
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С	56 T-T1	20 92	010	1.643	0.175	2 0									
С	47.174	46.967													
С	34.738	37.539													
	57 T1-R2	92 90	050	0.002	0.000	1 0									
С	48.799														
С	41.637														
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С	50.174														
C	46.367	02 04	0.50	0.003	0.000	1.0									
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c	47.209														
	60 R2-U2	90 94	020	0.815	0.002	7.0									
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С	47.146														
С	47.251														
	62 V-V1	22 95	010	1.078	0.134	2 0									
С	47.818	46.682													
C	51.129 62.V1.V2	52.387	0.50	0.003	0.146	1.0									
C	63 V1-V2 45.493	95 96	050	0.002	0.146	1 0									
c	52.860														
	64 V2-W2	96 98	020	1.040	0.202	5 0									
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С	43.775														
С	56.962														
	66 W1-W2	97 98	050	0.002	0.000	10									
С	43.550														
С	58.852	00 105		4.0==											
	67 W2-X2 42.932	98 100	020 754 42	1.675		42.609	12 522	42.635	42.581	42.573	42.614	42.715	42.750	42.771	42.767
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С		24 99	010	0.951	0.194	3 0									
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	68 X-X1 42.875	41.554 40.8													
С	68 X-X1 42.875 64.959	67.089 68.0	165	0.003	0.000	1.0									
C C	68 X-X1 42.875 64.959 69 X1-X2			0.002	0.000	1 0									
c c	68 X-X1 42.875 64.959 69 X1-X2 40.463	67.089 68.0	165	0.002	0.000	1 0									
C C C	68 X-X1 42.875 64.959 69 X1-X2	67.089 68.0	165	0.002 3.205		10									



39.764 38.004 36.691 35.539 34.651

72.562 76.037 78.553 80.470 86.350 39.651 37.949 36.518 35.458 34.595

72.824 76.132 78.689 80.547 86.421 39.619 37.891 36.375 35.421 34.538

72.924 76.211 78.920 80.607 86.439 39.522 37.778 36.343 35.230 34.517

73.414 76.363 78.992 81.225 86.521

C 40.454	40.412 40.387	40.355	40 212	40 242	40 10E	40 1E6	40 141	40.067	39.889
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	440 37.196 37								
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		.499 79.560							
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C 71 Y-Y1 C 35.825	25 101 35.367 35.052	010 1.269		5 0					
	84.292 86.507								
C 72 Y1-Y2	101 102	0 5 0 0.002	0.893	10					
C 33.780 C 88.828									
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	37.802 38.165				39.367				
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C 39.044									
C 75 Z2-AA2			9 0.10	3 3 0					
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C 76 AA-AA1			6 0.16	8 5 0					
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	39.093 39.243 2 105 106			EG 10					
C 33.313	2 103 100	030 0.0	102 0.3	36 10					
C 42.023									
C 78 AA2-AB	2 106 108 33.540 33.575	020 0.9	91 0.2	30 5 0					
C 33.396 C 42.207	42.302 42.554	44.881	5.137						
C 79 AC-AC1	28 109	0 1 0 2.01	3 0.111						
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C 36.325	36.226 35.681	35.071	4.772						
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C 81 AB-AB1 C 35.412	29 107	010 0.54	4 0.12	8 10					
C 48.560									
C 82 AB1-AB	2 107 108	050 0.0	0.0	00 1 0					
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C 30.216 C 57.372									
C 90 AF-AF1		010 0.82	5 0.271	L 2 0					
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	33 116	010 0.52	26 0.07	3 1 0					
C 35.679 C 54.993									
C 93 AG1-AG	2 116 117	050 0.0	0.0	30 1 0					
C 34.497									
C 55.434 C 94 AG2-AD	3 117 112	020 0.7	748 0.0	02 1 0					
C 31.958			_ 5.0						
C 52.794	144 440	020 22	42 00	71 4 0					
	2 114 119 29.886 29.798		45 0.0	/1 4 U					
C 57.565	57.605 57.645	57.738							
C 96 AH-AH1	34 118	020 1.45			27.025	37.004	47-	20.000	20.420
	27.879 27.718 50.047 51.461								
	2 118 119					-35-	3.040	3550	
C 29.507									
C 57.872 C 98 AH2-AI2	119 121	020 11	07 0.27	76 25 0					
All									



C 29.623 29.606 29.539	29.434 29.351 29.246	28.924	28.878	28.842	28.824	28.693	28.567	28.524	28.429	28.384
28.333 28.202 28.201 28.1 C 57.990 58.044 58.186	58.515 58.939 59.291	28.050 60.887	28.046 61.163	27.984 61.283	61.382	61.786	62.399	62.556	63.036	63.484
63.779 64.217 64.237 64.2 C 99 Al-Al1 35 120 0	255 64.361 64.680 64.863 010 0.740 0.224 1 0	64.869	65.233	65.548						
C 29.474										
C 63.773 C 100 Al1-Al2 120 121	050 0.002 0.287 1 0									
C 28.072 C 65.726										
C 101 AI2-AJ2 121 123	020 1.767 0.180 55 0									
C 28.062 28.025 27.936 27.836 28.127 28.214 28.2	27.690 27.662 27.647 265 28.297 28.351 28.503	27.645 28.582	27.680 28.620	27.749 28.732	27.804 28.843	27.806 28.840	27.791 28.812	27.885 28.704	27.764 28.620	27.762 28.500
28.377 28.333 28.161 28.0	097 28.059 28.023 28.109	28.068	27.863	27.810	27.788	27.726	27.632	27.623	27.690	27.698
27.683 27.713 27.740 27.7 C 66.036 66.168 66.278		28.076 66.650	66.765	66.898	67.060	67.247	67.622	68.237	68.569	68.644
68.756 68.991 69.081 69.1 72.599 72.651 72.789 72.9		70.280 74.203	70.348 74.518	70.660 74.693	71.225 74.838	71.422 75.022	71.642 75.215	71.902 75.312	72.242 75.582	72.399 75.804
76.034 76.209 76.272 76.3		76.636	74.518	74.093	74.030	75.022	75.215	75.312	/3.362	75.804
C 102 AJ-AJ1 36 122 C 27.214 27.370	010 1.065 0.181 2 0									
C 72.460 76.157										
C 103 AJ1-AJ2 122 123 C 28.099	050 0.002 0.000 1 0									
C 76.759 C 104 AJ2-AM3 123 128	0 3 0 0.032 0.014 1 0									
C 28.153	030 0.032 0.014 1 0									
C 76.884 C 105 AK-AK1 37 124	010 0.904 0.179 1 0									
C 34.190	010 0.501 0.175 10									
C 60.951 C 106 AK1-AM2 124 126	020 0.774 0.247 10									
C 30.966 C 68.049										
C 107 AL-AL1 39 125	010 0.529 0.208 1 0									
C 32.059 C 67.535										
C 108 AL1-AM2 125 126	050 0.002 0.102 10									
C 30.889 C 68.027										
C 109 AM-AM1 38 127 C 31.360 30.868 30.858	010 1.214 0.241 3 0									
C 61.691 66.848 67.468										
C 110 AM1-AM2 127 126 C 30.823	050 0.002 0.299 1 0									
C 67.947 C 111 AM2-AM3 126 128	0 2 0 1.318 0.230 4 0									
C 30.807 30.672 30.564	30.326									
C 68.163 69.025 69.547 C 112 AM3-AN2 128 134	70.364 0 2 0 0.852 0.002 25 0									
C 28.225 28.222 28.208 28.543 28.643 28.893 29.0	28.100 28.154 28.207 036 29.082 29.109 29.121	28.244 29.120	28.274 29.131	28.269 29.216	28.254	28.210	28.174	28.179	28.207	28.405
C 77.255 77.437 77.526	77.888 78.086 78.197	78.317	78.504	78.852	78.965	79.157	79.424	79.600	79.725	79.995
80.136 80.179 80.222 80.2 C 113 AN-AN1 40 129	285 80.359 80.445 80.535 0 1 0 0.852 0.122 1 0	80.725	80.774	80.875						
C 30.029 C 78.233										
C 114 AN1-AN2 129 134	050 0.002 0.000 10									
C 29.928 C 81.465										
C 115 AO-AO1 41 130 C 38.318 38.485	010 0.963 0.209 2 0									
C 64.965 67.370										
C 116 AO1-AP2 130 132 C 38.303 37.942 37.600		36.371	36.141	35.905	35.616	35.248	34.952	34.676	34.164	34.072
33.710 33.645 33.152 C 69.259 69.172 69.281		69.758		70.524			74 444			
72.660 72.780 72.671		03.738	70.185	70.324	70.853	71.105	71.411	71.981	71.521	71.554
C 117 AP-AP1 42 131 C 34.126	010 0.812 0.194 1 0									
C 70.350	0.50 0.003 0.440 4.5									
C 118 AP1-AP2 131 132 C 32.945	050 0.002 0.440 10									
C 72.755 C 119 AQ-AQ1 43 133	010 1.066 0.168 2 0									
C 36.256 35.413										
C 72.613 72.818 C 120 AQ1-AP2 133 132	050 0.002 0.267 1 0									
C 32.954 C 72.778										
C 121 AP2-AN2 132 134		24 400	20.044	20 72-	20 500	20.551	20.522	20.545	20.425	20.400
30.359 30.143 29.911 29.8	31.620 31.603 31.366 856 29.829 29.831 29.842	29.917	30.053	30.103	30.093	30.020	29.976		30.424	30.406
	75.130 75.183 75.609 059 79.474 79.857 80.000						76.760 81.391	76.917	78.398	78.502
C 122 AR-AR1 44 135										
C 31.541 30.701 C 78.498 79.337										
C 123 AR1-AN2 135 134 C 29.967	050 0.002 0.138 1 0									
C 81.442	040 400 0									
C 45.516	010 1.095 0.147 1 0									
C 70.271										



_																
C	125 AS1-AT2 43.103	13	6 138	010	0.928	0.382	1 0									
c																
	126 AT-AT1		137	010	0.784	0.435	2 0									
C C		41.851 76.332														
	127 AT1-AT2		7 138	050	0.002	0.271	1 0									
c	41.801															
С	78.110															
	128 AT2-AU2	13	38 140	010	0.803	0.280	1 0									
C C	40.953 80.887															
	129 AU-AU1	4	7 139	010	0.966	0.328	1 0									
c	40.269															
C	79.861															
C	130 AU1-AU2 40.101	1	39 140	050	0.002	0.184	1 0									
c	83.541															
	131 AU2-AV2	14	40 142	020	0.643	0.192	4 0									
С			39.829	40.326												
С		84.932		86.670												
C	132 AV-AV1 39.787	48	3 141	010	0.895	0.272	1 0									
C	84.589															
	133 AV1-AV2	14	11 142	050	0.002	0.000	1 0									
С	40.496															
C	87.794															
	134 AV2-AW2 40.501	40.398	.42 144 40.445	020	0.428	0.002	3 0	,								
C C		40.398 89.225	40.445 90.852													
	135 AW-AW1		49 143	010	1.047	0.224	1 0									
С	38.978															
C	87.955							•								
c	136 AW1-AW2 40.508		143 144	050	0.002	0.03	3 1	U								
c	91.042															
	137 AW2-BB2	1	44 153	020	0.339	0.022	6 0									
С	40.640		40.796	41.113	41.189	41.3										
С	91.128		91.561	92.050	92.205	92.8										
C	138 AX-AX1 48.391	50	145	010	0.645	0.174	1 0									
c	72.968															
	139 AX1-AY2	14	IS 147	010	1.204	0.303	4 0									
С	46.855	45.749	45.709	45.882												
C		79.692	81.380	83.068												
C	140 AY-AY1 48.093	47.493	146	010	1.038	0.276	2 0									
c		81.383														
	141 AY1-AY2		6 147	050	0.002	0.334	1 0									
С	45.914															
C	84.199		7 440	0.20	0.226	0.430										
C	142 AY2-AZ2 45.835	14	7 149	020	0.226	0.139	1 0									
c	85.515															
С	143 AZ-AZ1		148	010	1.217	0.213	2 0									
C	48.253	47.573														
C	83.426 144 AZ1-AZ2	84.648	8 149	050	0.002	0.124	1 0									
c	45.810	14	0 143	030	0.002	0.124	10									
С	85.911															
	145 AZ2-BA2		9 151	020	0.538	0.215										
c		45.747 86.282		45.627 87.176	45.589 87.604	45.5 87.9		45.522 88.061	45.463 88.348	45.434 88.405	45.326 88.560	45.300 88.574	44.822 89.215			
	146 BA-BA1	53		010		0.138		88.001	88.348	88.403	88.300	00.374	65.215			
c	44.561															
C																
	147 BA1-BA2	15	50 151	050	0.002	0.269	1 0									
C C																
	148 BA2-BB2	15	1 153	020	0.880	0.184	19 0									
С				43.457	43.248	8 43	.102	43.060	43.007	42.970	42.948	42.937	42.894	42.600	42.259	42.043
	.994 41.768		579 41.5													
C	89.423 2.183 92.508		89.891 500 92.8	90.154	90.323	3 90	.545	90.630	90.764	90.896	91.181	91.234	91.296	91.519	91.872	92.137
	149 BB-BB1		152		0.596	0.116	1 0									
c																
С																
	150 BB1-BB2	15	2 153	050	0.002	0.049	1 0									
c																
	92.829 151 BB2-BB3	15	3 154	020	0.186	0.065	4 0									
c	41.470	41.397	41.280	41.224			-									
C			93.802	94.010			_	_								
	152 AN2-OUTI	.ET	134 155	050	0.000	0.00	0 1	0								
C C	29.863 88.407															
	153 Y2-OUTLE	т 1	02 155	050	0.000	0.000	1 0)								
С	31.740															
C			154 45-	0.5.0	0.000			•								
C	154 BB3-OUTL 35.484	EI	154 155	050	0.000	0.00	0 1	U								
c	94.793															
С																
C	#STORAGES															



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С
C#INFLOW/OUTFLOW
C END RORB_GE
C File created using MiRORB version 1.2
C Original CATG file created on 27/05/2014 at 15:41:32
                                                           Sub-area A, Reach A-A1 - Generate rainfall excess h'graph and route downstream
1. 1. 1.402. -99
                                .Reach 1 node 1
5, 1, .095, -99
                                ,Reach 2
                                                       Reach A1-B2 - Route running h'graph downstream
                                              Store running hydrograph
                                                          Sub-area B, Reach B-B1 - Generate rainfall excess h'graph and route downstream
1, 1, .960, -99
                               ,Reach 3 node 2
                                              Add running h'graph to last stored h'graph
Reach B2-B3 - Route running h'graph downstream
5, 1, .224, -99
                                Reach 5
5, 1, .170, -99
                                Reach 8
                                                       Reach B3-C2 - Route running h'graph downstream
                                              Store running hydrograph
                                                           Sub-area C, Reach C-C1 - Generate rainfall excess h'graph and route downstream
1, 1, 1.166, -99
                                              Add running h'graph to last stored h'graph
                                              PRINT
KennedyCkEUpper_C2
                               ,Reach 9
5, 1, .332, -99
                                                       Reach C2-D2 - Route running h'graph downstream
                                              Store running hydrograph
1, 1, .762, -99
                                .Reach 10 node 4
                                                           Sub-area D, Reach D-D1 - Generate rainfall excess h'graph and route downstream
                                              Add running h'graph to last stored h'graph
5, 1, .225, -99
                                Reach 12,
                                                       Reach D2-D3 - Route running h'graph downstream
5, 2, .942, .318, -99
                                 .Reach 13
                                                         Reach D3-E2 - Route running h'graph downstream
                                              Store running hydrograph
1, 1, 1.168, -99
                                Reach 14 node 5
                                                           Sub-area E, Reach E-E1 - Generate rainfall excess h'graph and route downstream
                                              Add running h'graph to last stored h'graph
                                                          Reach E2-F2 - Route running h'graph downstream
5, 2, .850, .002, -99
                                 ,Reach 16
                                              Store running hydrograph
1, 1, 1.580, -99
                                Reach 17 node 6
                                                           Sub-area F, Reach F-F1 - Generate rainfall excess h'graph and route downstream
                                              Add running h'graph to last stored h'graph
                                                          Reach F2-J3 - Route running h'graph downstream
5, 2, .202, .125, -99
                                  ,Reach 30
                                              Store running hydrograph
1. 2. .526. .108. -99
                                                          Sub-area K, Reach K-K1 - Generate rainfall excess h'graph and route downstream Reach K1-L2 - Route running h'graph downstream
                                  Reach 31 node 11
5, 2, 1.290, .423, -99
                                  ,Reach 32
                                              Store running hydrograph
                                Reach 33 node 12
                                                           Sub-area L. Reach L-L1 - Generate rainfall excess h'graph and route downstream
1. 1. .621. -99
                                              Add running h'graph to last stored h'graph
5, 2, .756, .192, -99
                                  Reach 35
                                                          Reach L2-M2 - Route running h'graph downstream
                                              Store running hydrograph
1, 1, .793, -99
                                Reach 36 node 13
                                                           Sub-area M, Reach M-M1 - Generate rainfall excess h'graph and route downstream
                                              Add running h'graph to last stored h'graph
                                              PRINT
KennedyCkWUpper_M2
                                                          Reach M2-N2 - Route running h'graph downstream
5, 2, .796, .177, -99
                                 ,Reach 38
                                              Store running hydrograph
1. 2. .453. .222. -99
                                 .Reach 39 node 14
                                                              Sub-area N, Reach N-N1 - Generate rainfall excess h'graph and route downstream
                                              Add running h'graph to last stored h'graph
5, 2, .892, .144, -99
                                  Reach 41
                                                          Reach N2-O2 - Route running h'graph downstream
                                              Store running hydrograph
1, 1, 1.115, -99
                                Reach 42 node 15
                                                            Sub-area O, Reach O-O1 - Generate rainfall excess h'graph and route downstream
                                              Add running h'graph to last stored h'graph
5, 2, .270, .002, -99
                                                          Reach O2-J2 - Route running h'graph downstream
                                 ,Reach 44
                                              Store running hydrograph
                                                          Sub-area G, Reach G-G1 - Generate rainfall excess h'graph and route downstream
1.1..660. -99
                                .Reach 19 node 7
5, 1, 1.009, -99
                                                        Reach G1-H2 - Route running h'graph downstream
                                ,Reach 20
                                              Store running hydrograph
1, 1, .679, -99
                                Reach 21 node 8
                                                           Sub-area H, Reach H-H1 - Generate rainfall excess h'graph and route downstream
                                              Add running h'graph to last stored h'graph
5. 1. 1.501. -99
                                .Reach 23
                                                        Reach H2-I2 - Route running h'graph downstream
                                              Store running hydrograph
1, 2, .723, .334, -99
                                  Reach 24 node 9
                                                             Sub-area I, Reach I-I1 - Generate rainfall excess h'graph and route downstream
                                              Add running h'graph to last stored h'graph
KennedvCkMUpper 12
5, 2, 2.278, .236, -99
                                  ,Reach 26
                                                          Reach I2-J2 - Route running h'graph downstream
                                              Add running h'graph to last stored h'graph
                                              Store running hydrograph
1, 1, 1.238, -99
                                Reach 27 node 10
                                                            Sub-area J, Reach J-J1 - Generate rainfall excess h'graph and route downstream
                                              Add running h'graph to last stored h'graph
Reach J2-J3 - Route running h'graph downstream
5, 2, .017, .078, -99
                                  Reach 29
                                              Add running h'graph to last stored h'graph
                                              PRINT
KennedvCkHilbrickRd J3
5, 2, 1.230, .263, -99
                                  Reach 45,
                                                          Reach J3-J4 - Route running h'graph downstream
                                              Store running hydrograph
1. 1. 2.202. -99
                                ,Reach 46 node 17
                                                            Sub-area Q, Reach Q-Q1 - Generate rainfall excess h'graph and route downstream
                                              Add running h'graph to last stored h'graph
Reach J4-P2 - Route running h'graph downstream
5, 2, .104, .404, -99
                                 ,Reach 48
                                              Store running hydrograph
                                                           Sub-area P, Reach P-P1 - Generate rainfall excess h'graph and route downstream
1, 1, .949, -99
                                Reach 49 node 16
                                              Add running h'graph to last stored h'graph
                                 .Reach 51
5. 3. .679. .151. -99
                                                          Reach P2-R2 - Route running h'graph downstream
                                              Store running hydrograph
1, 1, 1.493, -99
                                Reach 52 node 18
                                                            Sub-area R, Reach R-R1 - Generate rainfall excess h'graph and route downstream
                                              Add running h'graph to last stored h'graph
                                              Store running hydrograph
1, 1, 1.312, -99
                                Reach 54 node 19
                                                            Sub-area S, Reach S-S1 - Generate rainfall excess h'graph and route downstream
                                              Add running h'graph to last stored h'graph
                                              Store running hydrograph
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1, 1, 1.643, -99	,Reach 56 node 20 Sub-area T, Reach T-T1 - Generate rainfall excess h'graph and route downstream
4 ,	Add running h'graph to last stored h'graph PRINT
, R2	
5, 2, .815, .002, -99 3 ,	,Reach 60 Reach R2-U2 - Route running h'graph downstream Store running hydrograph
1, 1, 1.082, -99 7	,Reach 58 node 21 Sub-area U, Reach U-U1 - Generate rainfall excess h'graph and route downstream PRINT
U1 ,	
4 , 5, 2, .825, .083, -99	Add running h'graph to last stored h'graph ,Reach 61 Reach U2-V2 - Route running h'graph downstream
3 , 1, 1, 1.078, -99	Store running hydrograph ,Reach 62 node 22 Sub-area V, Reach V-V1 - Generate rainfall excess h'graph and route downstream
7 V1	PRINT
4 , 5, 2, 1.040, .202, -99	Add running h'graph to last stored h'graph ,Reach 64 Reach V2-W2 - Route running h'graph downstream
3 ,	Store running hydrograph ,Reach 65 node 23 Sub-area W, Reach W-W1 - Generate rainfall excess h'graph and route downstream
1, 1, .504, -99 7 ,	PRINT
W1 4 ,	Add running h'graph to last stored h'graph
7.2 , KennedyCkRailwayLine_W2	PRINT
5, 2, 1.675, .174, -99	,Reach 67 Reach W2-X2 - Route running h'graph downstream
3 , 1, 1, .951, -99	Store running hydrograph ,Reach 68 node 24 Sub-area X, Reach X-X1 - Generate rainfall excess h'graph and route downstream
7 , X1	PRINT
4 ,	Add running h'graph to last stored h'graph ,Reach 70 Reach X2-Y2 - Route running h'graph downstream
5, 2, 3.205, .137, -99 3 ,	Store running hydrograph
1, 1, 1.269, -99 7 ,	Reach 71 node 25 Sub-area Y, Reach Y-Y1 - Generate rainfall excess h'graph and route downstream, PRINT
Y1 4	Add running h'graph to last stored h'graph
7.2 ,	PRINT
KennedyCkOutlfall_Y2 3 ,	Store running hydrograph
1, 1, 1.018, -99 7 ,	Reach 73 node 26 Sub-area Z, Reach Z-Z1 - Generate rainfall excess h'graph and route downstream, PRINT
Z2 5, 2, .749, .103, -99	,Reach 75 Reach Z2-AA2 - Route running h'graph downstream
3 , 1, 2, .526, .168, -99	Store running hydrograph ,Reach 76 node 27 Sub-area AA, Reach AA-AA1 - Generate rainfall excess h'graph and route downstream
7 ,	PRINT
AA1 4 ,	Add running h'graph to last stored h'graph
5, 2, .991, .230, -99 3	,Reach 78 Reach AA2-AB2 - Route running h'graph downstream Store running hydrograph
1, 1, 2.013, -99 7	Reach 79 node 28 Sub-area AC, Reach AC-AC1 - Generate rainfall excess h'graph and route downstream, PRINT
AC1	
5, 2, .784, .175, -99 4 ,	,Reach 80 Reach AC1-AB2 - Route running h'graph downstream Add running h'graph to last stored h'graph
3 , 1, 1, .544, -99	Store running hydrograph ,Reach 81 node 29 Sub-area AB, Reach AB-AB1 - Generate rainfall excess h'graph and route downstream
7 , AB1	PRINT
4 ,	Add running h'graph to last stored h'graph
7.2 , BenellaRailwayLine_AB2	PRINT
5, 2, .469, .002, -99 3	,Reach 83 Reach AB2-AD2 - Route running h'graph downstream Store running hydrograph
1, 1, .739, -99 4	,Reach 84 node 30 Sub-area AD, Reach AD-AD1 - Generate rainfall excess h'graph and route downstream
5, 2, .565, .078, -99	Add running h'graph to last stored h'graph ,Reach 86 Reach AD2-AD3 - Route running h'graph downstream
3 , 1, 1, .825, -99	Store running hydrograph ,Reach 90 node 32 Sub-area AF, Reach AF-AF1 - Generate rainfall excess h'graph and route downstream
7 , AF1	PRINT
5, 2, .619, .076, -99 3 ,	,Reach 91 Reach AF1-AG2 - Route running h'graph downstream Store running hydrograph
1, 1, .526, -99 7	,Reach 92 node 33 Sub-area AG, Reach AG-AG1 - Generate rainfall excess h'graph and route downstream
AG1	
4 , 5, 2, .748, .002, -99	Add running h'graph to last stored h'graph ,Reach 94 Reach AG2-AD3 - Route running h'graph downstream
4 , 5, 2, .652, .084, -99	Add running h'graph to last stored h'graph ,Reach 87 Reach AD3-AE2 - Route running h'graph downstream
3 , 1, 1, 1.086, -99	Store running hydrograph ,Reach 88 node 31 Sub-area AE, Reach AE-AE1 - Generate rainfall excess h'graph and route downstream
4 ,	Add running h'graph to last stored h'graph
5, 3, .143, .071, -99 3 ,	,Reach 95 Reach AE2-AH2 - Route running h'graph downstream Store running hydrograph
1, 2, 1.451, .136, -99 4 ,	,Reach 96 node 34 Sub-area AH, Reach AH-AH1 - Generate rainfall excess h'graph and route downstream Add running h'graph to last stored h'graph
7.2 , BenellaRacecourseRd_AH2	PRINT
5, 2, 1.107, .276, -99	,Reach 98 Reach AH2-AI2 - Route running h'graph downstream
3 , 1, 1, .740, -99	Store running hydrograph ,Reach 99 node 35 Sub-area Al, Reach Al-Al1 - Generate rainfall excess h'graph and route downstream
4 , 5, 2, 1.767, .180, -99	Add running h'graph to last stored h'graph ,Reach 101 Reach Al2-AJ2 - Route running h'graph downstream
•	



3 , 1, 1, 1.065, -99	Store running hydrograph ,Reach 102 node 36 Sub-area AJ, Reach AJ-AJ1 - Generate rainfall excess h'graph and route downstream
7 AJ1	PRINT
4 ,	Add running h'graph to last stored h'graph
5, 3, .032, .014, -99 3	,Reach 104 Reach AJ2-AM3 - Route running h'graph downstream Store running hydrograph
1, 1, .904, -99	,Reach 105 node 37 Sub-area AK, Reach AK-AK1 - Generate rainfall excess h'graph and route downstream
7 , AK1	PRINT
5, 2, .774, .247, -99	,Reach 106 Reach AK1-AM2 - Route running h'graph downstream
3 , 1, 1, 1.214, -99	Store running hydrograph ,Reach 109 node 38 Sub-area AM, Reach AM-AM1 - Generate rainfall excess h'graph and route downstream
7 , AM1	PRINT
4 ,	Add running h'graph to last stored h'graph
3 , 1, 1, .529, -99	Store running hydrograph ,Reach 107 node 39 Sub-area AL, Reach AL-AL1 - Generate rainfall excess h'graph and route downstream
7 ,	PRINT
AL1 4 .	Add running h'graph to last stored h'graph
5, 2, 1.318, .230, -99	,Reach 111 Reach AM2-AM3 - Route running h'graph downstream
4 , 7.2 ,	Add running h'graph to last stored h'graph , PRINT
BenellaNorthRd_AM3	Doob 442
5, 2, .852, .002, -99 3 ,	,Reach 112 Reach AM3-AN2 - Route running h'graph downstream Store running hydrograph
1, 1, .963, -99 7	,Reach 115 node 41 Sub-area AO, Reach AO-AO1 - Generate rainfall excess h'graph and route downstream
A01	
5, 1, 1.470, -99 3	,Reach 116 Reach AO1-AP2 - Route running h'graph downstream Store running hydrograph
1, 1, .812, -99	,Reach 117 node 42 Sub-area AP, Reach AP-AP1 - Generate rainfall excess h'graph and route downstream
7 , AP1	PRINT
4 ,	Add running h'graph to last stored h'graph
3 , 1, 1, 1.066, -99	Store running hydrograph ,Reach 119 node 43 Sub-area AQ, Reach AQ-AQ1 - Generate rainfall excess h'graph and route downstream
7 , AQ1	PRINT
4 ,	Add running h'graph to last stored h'graph
5, 2, 1.464, .186, -99 4	,Reach 121 Reach AP2-AN2 - Route running h'graph downstream Add running h'graph to last stored h'graph
3 ,	Store running hydrograph
1, 1, .852, -99 7 ,	,Reach 113 node 40 Sub-area AN, Reach AN-AN1 - Generate rainfall excess h'graph and route downstream PRINT
AN1 4	Add moving blown has look should blown h
3 ,	Add running h'graph to last stored h'graph Store running hydrograph
1, 1, 1.007, -99 7	,Reach 122 node 44 Sub-area AR, Reach AR-AR1 - Generate rainfall excess h'graph and route downstream PRINT
AR1	FAINI
4 , 7.2 ,	Add running h'graph to last stored h'graph , PRINT
BenellaOutlet_AN2	
4 , 3 ,	Add running h'graph to last stored h'graph Store running hydrograph
1, 1, 1.095, -99	,Reach 124 node 45 Sub-area AS, Reach AS-AS1 - Generate rainfall excess h'graph and route downstream
5, 1, .928, -99 3 ,	,Reach 125 Reach AS1-AT2 - Route running h'graph downstream Store running hydrograph
1, 1, .784, -99 4	,Reach 126 node 46 Sub-area AT, Reach AT-AT1 - Generate rainfall excess h'graph and route downstream Add running h'graph to last stored h'graph
7 ,	PRINT
AT2 5, 1, .803, -99	,Reach 128 Reach AT2-AU2 - Route running h'graph downstream
3 ,	Store running hydrograph
1, 1, .966, -99 7 ,	,Reach 129 node 47 Sub-area AU, Reach AU-AU1 - Generate rainfall excess h'graph and route downstream PRINT
AU1 4 ,	Add running h'graph to last stored h'graph
7.2 ,	Add running n graph to last stored n graph PRINT
EastTribLinkRdWest_AU2 5, 2, .643, .192, -99	,Reach 131 Reach AU2-AV2 - Route running h'graph downstream
3 ,	Store running hydrograph
1, 1, .895, -99 7 ,	Reach 132 node 48 Sub-area AV, Reach AV-AV1 - Generate rainfall excess h'graph and route downstream, PRINT
AV1	Add more to a bloom by the last store of bloom by
4 , 5, 2, .428, .002, -99	Add running h'graph to last stored h'graph ,Reach 134 Reach AV2-AW2 - Route running h'graph downstream
3 , 1, 1, 1.047, -99	Store running hydrograph ,Reach 135 node 49 Sub-area AW, Reach AW-AW1 - Generate rainfall excess h'graph and route downstream
7 ,	PRINT
AW1 4 .	Add running h'graph to last stored h'graph
5, 2, .339, .022, -99	,Reach 137 Reach AW2-BB2 - Route running h'graph downstream
3 , 1, 1, .645, -99	Store running hydrograph ,Reach 138 node 50 Sub-area AX, Reach AX-AX1 - Generate rainfall excess h'graph and route downstream
5, 1, 1.204, -99 3 ,	,Reach 139 Reach AX1-AY2 - Route running h'graph downstream Store running hydrograph
1, 1, 1.038, -99	,Reach 140 node 51 Sub-area AY, Reach AY-AY1 - Generate rainfall excess h'graph and route downstream
4 , 5, 2, .226, .139, -99	Add running h'graph to last stored h'graph ,Reach 142 Reach AY2-AZ2 - Route running h'graph downstream
3 ,	Store running hydrograph
1, 1, 1.217, -99	Reach 143 node 52 Sub-area AZ, Reach AZ-AZ1 - Generate rainfall excess h'graph and route downstream,



```
Add running h'graph to last stored h'graph
7.2
                                                 PRINT
EastTribLinkRdEast_AZ2
5, 2, .538, .215, -99
                                   ,Reach 145
                                                             Reach AZ2-BA2 - Route running h'graph downstream
                                               Store running hydrograph
ode 53 Sub-area BA, Reach BA-BA1 - Generate rainfall excess h'graph and route downstream
1, 1, .721, -99
                                 ,Reach 146 node 53
                                               Add running h'graph to last stored h'graph
Reach BA2-BB2 - Route running h'graph downstream
5. 2. .880. .184. -99
                                  .Reach 148
                                                Add running h'graph to last stored h'graph
                                                Store running hydrograph
ide 54 Sub-area BB, Reach BB-BB1 - Generate rainfall excess h'graph and route downstream
                                 ,Reach 149 node 54
1, 1, .596, -99
                                                Add running h'graph to last stored h'graph
5, 2, .186, .065, -99
                                  Reach 151
                                                            Reach BB2-BB3 - Route running h'graph downstream
                                                 PRINT
EastTribOutlet_BB3
                                                Add running h'graph to last stored h'graph
C Sub-area areas in km2
 1.179, 0.699, 0.955, 0.683, 1.028,
 1.268, 0.460, 1.057,
                           1.046.
                                   0.574.
 1.116, 0.782, 0.694, 0.655,
                                   0.804,
 1.158, 0.988,
                   1.247,
                           0.910,
                                    1.061,
 1.195, 1.063,
1.358, 0.814,
                                    1.015.
                   0.961,
                           1.189,
                   1.577, 0.762,
                                   0.852,
 0.746, 0.514,
1.278, 0.670,
                  1.114, 1.175,
0.547, 0.711,
                                   0.799,
1.261,
 0.737, 0.949,
                  0.819, 0.835,
 0.919. 0.883. 0.701.
                           1.082,
                                   0.643,
 0.977, 0.604, 0.613, 0.881,
C Impervious Fraction Data
 0.050, 0.050, 0.050, 0.048, 0.050,
 0.050, 0.050, 0.050, 0.050,
                                   0.050,
 0.050, 0.050,
0.050, 0.050,
                                   0.050,
0.099,
                   0.050,
                           0.050,
                  0.129,
                           0.075,
 0.055, 0.054,
                   0.114,
                           0.050,
 0.100, 0.424,
0.150, 0.150,
                          0.434,
0.200,
                   0.245.
                                    0.538.
         0.150,
                   0.150,
                                    0.145,
                  0.172, 0.231,
0.596, 0.599,
                                   0.095,
0.050,
 0.192,
         0.207,
 0.051. 0.193.
 0.050, 0.050,
                   0.050,
                           0.050,
                                   0.076,
 0.052, 0.050,
                   0.064,
                           0.078,
 -99
```